STRUCTURAL GENERAL NOTES

MISCELLANEOUS

1. THESE ABBREVIATED DRAWING NOTES ARE WRITTEN TO MATCH THE BOOK SPECIFICATIONS. IF THERE ARE ANY ITEMS THAT DO NOT CORRESPOND EXACTLY AS WRITTEN, THE MORE STRINGENT WILL TAKE PRECEDENCE.

2. THE STRUCTURAL SYSTEM IS UNSTABLE UNTIL ALL CONNECTIONS HAVE BEEN MADE AND ALL CONCRETE HAS REACHED ITS MINIMUM DESIGN STRENGTH, AS SHOWN IN THE STRUCTURAL DOCUMENTS.

3. CONTRACTOR IS RESPONSIBLE FOR MEANS AND METHODS OF CONSTRUCTION TO ENSURE THE SAFETY OF THE BUILDING UNTIL STRUCTURAL SYSTEM IS COMPLETED. THIS INCLUDES, BUT IS NOT LIMITED TO, THE ADDITION OF WHATEVER TEMPORARY BRACING, SHORING, GUYS OR TIE_DOWNS THAT MAY BE NECESSARY. SUCH MATERIAL SHALL BE REMOVED AND SHALL REMAIN THE PROPERTY OF THE CONTRACTOR AFTER COMPLETION OF THE PROJECT.

4. CONTRACTOR TO SUPPORT, BRACE AND SECURE EXISTING STRUCTURE AS REQUIRED. CONTRACTOR IS SOLELY RESPONSIBLE FOR THE SAFETY OF THE BUILDING DURING CONSTRUCTION.

5. APPLICABLE BUILDING CODE: 2010 FLORIDA BUILDING CODE.

6. GRAVITY DESIGN LOADS:

AREA	SUPERIMPOSED LIVE LOAD	TOTAL DEAD LOAD
ROOF	30 PSF	25 PSF
SECOND FLOOR	100 PSF	96 PSF

7. WIND DESIGN CRITERIA:
ULTIMATE BASIC WIND SPEED: Vult = 215 MPH (3 SECOND GUST)
EQUIVALENT NOMINAL BASIC WIND SPEED: Vasd = 170 MPH (3 SECOND GUST)
RISK CATEGORY = IV
EXPOSURE = C
ENCLOSED BUILDING INTERNAL PRESSURE COEFFICIENT, GCPI= +/-0.18 COMPONENTS AND CLADDING WIND PRESSURES = SEE TABLES
WIND BORNE DEBRIS REGION

8. ALL MATERIALS AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE REFERENCED BUILDING CODE.

9. COORDINATE ALL DIMENSIONS AND ELEVATIONS WITH THE ARCHITECTURAL DRAWINGS. DO NOT SCALE DRAWINGS.

10. CONTACT ENGINEER WITH ANY QUESTIONS OR DISCREPANCIES FOUND ON DRAWINGS.

11. CONTRACTOR TO VERIFY ALL EXISTING DIMENSIONS, ELEVATIONS, AND CONDITIONS PRIOR TO BEGINNING CONSTRUCTION.

12. SUBMIT SHOP DRAWINGS AS REQUIRED HEREIN. ALLOW FOR TWO WEEKS REVIEW TIME AFTER RECEIPT OF SUBMITTALS BY THIS FIRM. ALL SUBMITTALS SHALL BE CHECKED AND SIGNED BY THE GENERAL CONTRACTOR AND SIGNED/SEALED BY THE DELEGATED ENGINEER, WHERE SPECIFIED HEREIN.

13. CONTRACTOR SHALL NOT BE RELIEVED FROM RESPONSIBILITY FOR ERRORS OR OMISSIONS IN SHOP DRAWINGS OR MIX DESIGNS BY THE ENGINEER'S REVIEW THEREOF.

14. ANY CHANGES TO THE STRUCTURE SHALL HAVE BEEN REVIEWED AND APPROVED IN WRITING BY THE ENGINEER PRIOR TO COMMENCING WORK ON ITEMS AFFECTED.

15. CONTRACTOR SHALL NOTIFY THIS OFFICE WHEN THE STRUCTURAL SYSTEM IS SUBSTANTIALLY COMPLETED, AND BEFORE SHEATHING, CEILINGS, OR ROOFING IS INSTALLED.

LEED SUBMITTALS

1. SUBMIT MANUFACTURER'S PRODUCT DATA WITH INFORMATION INCLUDING COMPLETE RECYCLED CONTENT INCLUDING PRE-CONSUMER AND POST-CONSUMER CONTENT PERCENTAGES. REFERENCE LEED NC VERSION 2009 REFERENCE GUIDE - MATERIALS AND RESOURCES CREDIT 4.1 & 4.2.
2. SUBMIT MANUFACTURER'S PRODUCT DATA WITH INFORMATION INCLUDING LOCATION OF EXTRACTION, HARVESTING, AND MANUFACTURING MILES FROM PROJECT. REFERENCE LEED NC VERSION 2009 REFERENCE GUIDE - MATERIALS AND RESOURCES CREDIT 5.1 & 5.2.
3. SUBMIT MANUFACTURER'S PRODUCT DATA WITH INFORMATION INCLUDING THE VOC LEVELS OF EACH PRODUCT. REFERENCE LEED NC VERSION 2009 REFERENCE GUIDE - INDOOR ENVIRONMENTAL QUALITY CREDITS 4.1, 4.2, 4.3, & 4.4.

HAND RAILS

1. AN ENGINEER REGISTERED IN THE STATE OF FLORIDA SHALL DESIGN RAILING SYSTEM AND CONNECTION OF IT TO THIS STRUCTURE.

2. SUBMIT SHOP DRAWINGS BEARING THE EMBOSSED SEAL AND THE SIGNATURE OF THE ENGINEER FOR REVIEW PRIOR TO FABRICATION.

3. THE CONFIGURATION OF THE RAILING SYSTEM SHALL BE AS SHOWN ON THE ARCHITECTURAL DRAWINGS.

4. RAILING SYSTEM AND CONNECTIONS SHALL BE DESIGNED FOR APPLICABLE LOADS AS INDICATED ON THE PLANS AND IN THE BUILDING CODE. THE LOADS SHALL BE CLEARLY INDICATED ON SHOP DRAWINGS.

5. SHOP DRAWINGS SHALL SHOW AND SPECIFY CONNECTIONS UTILIZED WITHIN THE RAILING SYSTEM AS WELL AS CONNECTIONS TO AND LOADS IMPOSED UPON THE STRUCTURAL SYSTEM SHOWN ON THESE PLANS.

GLAZED WINDOW AND DOOR SYSTEMS

1. EXTERIOR GLAZED OPENINGS SHALL BE DESIGNED IN ACCORDANCE WITH ACCEPTED ENGINEERING PRINCIPALS AND GOVERNING CODES.

2. THIS BUILDING HAS BEEN DESIGNED AS "ENCLOSED". REFER TO THE STRUCTURAL DRAWINGS FOR COMPONENTS AND CLADDING MINIMUM DESIGN WIND PRESSURES.

3. IF THIS SITE IS LOCATED WITHIN A WIND BORNE DEBRIS REGION, AS SHOWN IN FIGURE 1609 OF THE FLORIDA BUILDING CODE, ALL EXTERIOR GLAZED OPENINGS SHALL BE DESIGNED AND TESTED FOR MISSILE IMPACT OR PROTECTED BY APPROVED SCREENS OR SHUTTERS.

4. A LICENSED PROFESSIONAL ENGINEER SHALL DESIGN THE EXTERIOR GLAZED SYSTEMS, SHOP DRAWINGS, AND OVERSEE ANY LOAD TESTING.

5. SIGNED AND SEALED SHOP DRAWINGS, NOTICE OF ACCEPTANCE (NOA), OR FLORIDA PRODUCT APPROVAL DOCUMENTATION SHALL BE SUBMITTED IN ACCORDANCE WITH THE ARCHITECT'S REQUIREMENTS.

THRESHOLD BUILDINGS

1. IF THE BUILDING DEPARTMENT CLASSIFIES THIS PROJECT AS A THRESHOLD BUILDING, A SPECIAL INSPECTOR SHALL BE RETAINED IN ACCORDANCE WITH FLORIDA STATUTES.

2. THE SPECIAL INSPECTOR SHALL BE LICENSED BY THE FLORIDA BOARD OF PROFESSIONAL ENGINEERS AND SHALL STRICTLY FOLLOW THE "STRUCTURAL INSPECTION PLAN", PREPARED BY THIS OFFICE.

3. RESUMES OF THE SPECIAL INSPECTOR AND ANY OF HIS AUTHORIZED REPRESENTATIVES SHALL BE SUBMITTED TO THIS OFFICE FOR REVIEW. MCCARTHY RESERVES THE RIGHT TO REJECT ANY INSPECTOR THAT DOES NOT MEET OUR QUALIFICATIONS.

DELEGATED ENGINEER

1. WHERE NOTED HEREIN, A LICENSED PROFESSIONAL (DELEGATED) ENGINEER SHALL BE RETAINED TO DESIGN THE PRODUCT OR ASSEMBLY.

2. THE DELEGATED ENGINEER SHALL BE EXPERIENCED IN THE DESIGN OF THE REFERENCED PRODUCT OR ASSEMBLY.

3. IT IS THE DELEGATED ENGINEER'S RESPONSIBILITY TO REVIEW THE ENGINEER OF RECORD'S WRITTEN ENGINEERING REQUIREMENTS AND AUTHORIZATION FOR THE DELEGATED ENGINEERING DOCUMENT TO DETERMINE THE APPROPRIATE SCOPE OF ENGINEERING.

4. THE DELEGATED ENGINEERING DOCUMENT SHALL COMPLY WITH THE WRITTEN ENGINEERING REQUIREMENTS RECEIVED FROM THE ENGINEER OF RECORD. THEY SHALL INCLUDE THE PROJECT IDENTIFICATION AND THE CRITERIA USED AS A BASIS FOR ITS PREPARATION. IF A DELEGATED ENGINEER DETERMINES THERE ARE DETAILS, FEATURES OR UNANTICIPATED PROJECT LIMITS WHICH CONFLICT WITH THE WRITTEN ENGINEERING REQUIREMENTS PROVIDED BY THE ENGINEER OF RECORD, THE DELEGATED ENGINEER SHALL TIMELY CONTACT THE ENGINEER OF RECORD FOR RESOLUTION OF CONFLICTS.

5. THE DELEGATED ENGINEER SHALL FORWARD THE DELEGATED ENGINEERING DOCUMENT TO THE ENGINEER OF RECORD FOR REVIEW. ALL FINAL DELEGATED ENGINEERING DOCUMENTS REQUIRE THE IMPRESSED SEAL AND SIGNATURE OF THE DELEGATED ENGINEER AND INCLUDE:
A) DRAWINGS INTRODUCING ENGINEERING INPUT SUCH AS DEFINING THE CONFIGURATION OR STRUCTURAL CAPACITY OF STRUCTURAL COMPONENTS AND/OR THEIR ASSEMBLY INTO STRUCTURAL SYSTEMS.
B) CALCULATIONS.

SITE WORK

1. A SUBSURFACE INVESTIGATION HAS BEEN COMPLETED AT THE PROJECT SITE BY GFA INTERNATIONAL & TESTING. SOIL BORING LOGS AND SITE PREPARATION PROCEDURES ARE INCLUDED IN THE PROJECT SOILS REPORT, DATED JAN. 5, 2012 WHICH IS AN INTEGRAL PART OF THESE CONTRACT DOCUMENTS.

2. SITE WORK SHALL BE DONE IN STRICT ACCORDANCE WITH THE PROJECT SOILS REPORT.

3. IF THE BUILDING LOCATION ON THE SITE DIFFERS FROM THE BORING LOCATION PLAN IN THE REPORT, ADDITIONAL BORINGS MUST BE DONE WITH A NEW SOILS REPORT.

4. DESIGN SOIL BEARING PRESSURE = 2500 PSF.

5. A QUALIFIED TESTING LABORATORY SHALL BE RETAINED TO PERFORM THE FOLLOWING MINIMUM TESTS. REFER TO SOILS REPORT FOR ANY ADDITIONAL TESTING.
A) ONE DENSITY TEST FOR EACH 2,000 SQUARE FEET OF COMPACTED SUBGRADE AND COMPACTED FILL.
B) ONE DENSITY TEST AT EACH COLUMN FOOTING.
C) ONE DENSITY TEST PER 50 FEET OF WALL FOOTING.

6. ONE COPY OF ALL TEST REPORTS SHALL BE SENT DIRECTLY TO OWNER, ARCHITECT, STRUCTURAL ENGINEER, AND GENERAL CONTRACTOR.

7. FOUNDATION WALLS THAT RETAIN EARTH SHALL BE BRACED AGAINST BACKFILLING PRESSURES UNTIL FLOOR SLABS AT TOP AND BOTTOM ARE IN PLACE.

8. THE SIDES OF FOOTINGS SHALL BE FORMED WITH PLYWOOD.

9. EXERCISE CARE WHEN COMPACTING NEAR ADJACENT STRUCTURES. FOLLOW THE RECOMMENDATIONS IN THE SOILS REPORT AND DOCUMENT EXISTING CONDITIONS WITH PHOTOGRAPHS PRIOR TO STARTING WORK.

10. PRIOR TO CONSTRUCTION, CONTRACTOR SHALL LOCATE ALL EXISTING UNDERGROUND UTILITY LINES, TANKS, ETC. WITHIN THE CONSTRUCTION AREA AND RELOCATE THEM AS DIRECT BY THE CIVIL ENGINEER.

CAST IN PLACE CONCRETE

1. ALL CAST-IN-PLACE CONCRETE WORK INCLUDES REINFORCING STEEL AND RELATED WORK SHOWN INCLUDING FORMWORK, SETTING ANCHOR BOLTS, PLATES, FRAMES, DOWELS FOR MASONRY OR OTHER ITEMS EMBEDDED IN CONCRETE.

2. APPLICABLE STANDARDS

ACI NUMBER	TITLE
117	STANDARD SPECIFICATIONS FOR TOLERANCES FOR CONCRETE CONSTRUCTION
226	GROUND GRANULATED BLAST-FURNACE SLAG
301	STANDARD SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS
302	GUIDE FOR CONCRETE FLOOR AND SLAB CONSTRUCTION
304	GUIDE FOR MEASURING MIXING, TRANSPORTING AND PLACING CONCRETE
304.2R	PLACING CONCRETE BY PUMPING METHODS.
305R	HOT WEATHER CONCRETING
306R	COLD WEATHER CONCRETING
308	STANDARD PRACTICE FOR CURING CONCRETE
309R	GUIDE FOR CONSOLIDATION OF CONCRETE
315	MANUAL OF STANDARD PRACTICE FOR DETAILING CONCRETE STRUCTURES
318	BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE
347	RECOMMENDED PRACTICE FOR CONCRETE FORMWORK

CRSI NUMBER	TITLE
63	RECOMMENDED PRACTICE FOR PLACING REINFORCING BARS

3. CONCRETE MATERIALS
A) PORTLAND CEMENT - ASTM C 150, TYPE I
B) AGGREGATES - NORMAL WEIGHT CONCRETE, COARSE AND FINE, ASTM C33. STRUCTURAL LIGHT WEIGHT ASTM C330.
C) AIR-ENTRAINING - ASTM C260
D) WATER REDUCING - ASTM C494, TYPE A
E) WATER - FRESH, CLEAN AND POTABLE
F) NO ACCELERATORS, RETARDERS OR ADMIXTURES CONTAINING CHLORIDES WILL BE PERMITTED
G) FLY-ASH - ASTM C618, CLASS F, 20% MAXIMUM OF CEMENTITIOUS MATERIAL BY WEIGHT. DO NOT USE FOR EXPOSED SLABS OR ARCHITECTURAL CONCRETE.
H) SUPER PLASTICIZER - ASTM C494, TYPE F OR G, WHERE AUTHORIZED BY THE ENGINEER.
I) GROUND GRANULATED BLAST-FURNACE SLAG CEMENT - ASTM C989, 50% MAXIMUM BY WEIGHT.
J) MAXIMUM AGGREGATE SIZE - FOOTINGS = #57, OTHERS #67

5. PROVIDE THE FOLLOWING MINIMUM CONCRETE STRENGTHS AT 28 DAYS:
A) FOOTINGS, SLAB-ON-GRADE-----------------3000 PSI
B) APPARATUS BAY SLAB--------------------4000 PSI
C) REGULAR FILL ON PRECAST------------------3000 PSI
D) MASONRY WALL BEAMS, TIE COLUMNS-----------3000 PSI
E) FORMED COLUMNS, WALLS, BEAMS & SLABS-------4000 PSI

6. CONCRETE MUST BE BATCHED, MIXED AND TRANSPORTED IN ACCORDANCE WITH THE SPECIFICATIONS FOR READY-MIXED CONCRETE ASTM C94.

7. REQUIRED SLUMP = 4 PLUS OR MINUS ONE INCH.

8. CONCRETE MUST BE PLACED WITHIN 90 MINUTES OF BATCH TIME. WHEN AIR TEMPERATURE IS BETWEEN 85 AND 90 DEGREES F, REDUCE MIXING AND DELIVERY TIME TO 75 MINUTES. WHEN AIR TEMPERATURE IS HIGHER THAN 90 DEGREES F, REDUCE MIXING AND DELIVERY TIME TO 60 MINUTES.

9. DO NOT ADD WATER AT THE JOB SITE WITHOUT APPROVAL OF THE PROJECT SUPERINTENDENT. DO NOT EXCEED THE SLUMP LIMITATION. USE ONLY COLD WATER FROM THE TRUCK TANK. ANY ADDED WATER MUST BE INDICATED ON THE DELIVERY TICKET PLUS THE NAME OF THE PERSON AUTHORIZING. TEST CYLINDERS SHALL BE TAKEN AFTER THE ADDITION OF WATER.

10. LAP SPLICE REINFORCING PER 48 BAR DIAMETER TYPICAL MINIMUM UNLESS OTHERWISE SHOWN OR NOTED.

11. PROVIDE CORNER BARS AT ALL WALL FOOTING, WALL AND BEAM CORNERS. SIZE AND NUMBER TO MATCH HORIZONTAL BARS.

12. PROVIDE FOUNDATION DOWELS TO MATCH SIZE AND NUMBER OF VERTICAL BARS. EMBED DOWELS TO:
A) 3" ABOVE BOTTOM OF FOOTINGS

13. REINFORCEMENT SHALL BE FASTENED AND SECURED TOGETHER TO PREVENT DISPLACEMENT BY CONSTRUCTION LOADS OR THE PLACING OF CONCRETE.

14. REINFORCING BAR COVER
A) FOOTINGS 2" (TOP), 3" (SIDES AND BOTTOM)
B) COLUMNS AND BEAMS 1-1/2"
C) SLABS 3/4" (INTERIOR), 1-1/2" (EXTERIOR)

15. WHERE BAR LENGTHS ARE GIVEN ON THE DRAWINGS, LENGTH OF HOOK, IF REQUIRED, IS NOT INCLUDED.

16. SELECT PROPORTIONS IN ACCORDANCE WITH ACI 301 TO PROVIDE CONCRETE CAPABLE OF BEING PLACED WITHOUT EXCESSIVE SEGREGATION AND WITH ACCEPTABLE FINISHING PROPERTIES, DURABILITY, SURFACE HARDENERS, APPEARANCE, AND STRENGTH REQUIREMENTS REQUIRED BY THESE SPECIFICATIONS.

17. CHAIR WELDED WIRE FABRIC REINFORCING AT 3'-0" ON CENTER MAXIMUM IN EACH DIRECTION.

18. MAXIMUM WATER TO CEMENT RATIO WHEN NO BACK-UP DATA IS AVAILABLE:
A) 4000 PSI, 28-DAY COMPRESSIVE STRENGTH; W/C RATIO, 0.44 MAXIMUM (NON-AIR-ENTRAINED), 0.36 MAXIMUM (AIR-ENTRAINED).
B) 3000 PSI, 28-DAY COMPRESSIVE STRENGTH; W/C RATIO, 0.58 MAXIMUM (NON-AIR-ENTRAINED), 0.47 MAXIMUM (AIR-ENTRAINED).

19. DATA TO BE SUBMITTED:
A) INTENDED USAGE AND LOCATION FOR EACH TYPE
B) MIX DESIGN FOR EACH TYPE
C) CEMENT CONTENT IN POUNDS-PER-CUBIC YARD
D) COARSE AND FINE AGGREGATE IN POUNDS/CUBIC YARD
E) WATER CEMENT RATIO BY WEIGHT
F) CEMENT TYPE AND MANUFACTURER
G) SLUMP RANGE
H) AIR CONTENT
I) ADMIXTURE TYPE AND MANUFACTURER
J) PERCENT ADMIXTURE BY WEIGHT
K) STRENGTH TEST DATA REQUIRED TO ESTABLISH MIX DESIGN.
L) COMPLETE DETAIL AND PLACING SHOP DRAWINGS FOR ALL REINFORCING STEEL INCLUDING ACCESSORIES THAT HAVE BEEN REVIEWED AND STAMPED BY THE GENERAL CONTRACTOR. INCLUDE ALL REQUIRED DIMENSIONS AND ELEVATIONS (IE. TOP OF CONCRETE)

20. THE GENERAL CONTRACTOR IS RESPONSIBLE FOR PROVIDING THE CONSTRUCTION OF FORMWORK, SHORING AND RE-SHORING IN ACCORDANCE WITH ACI 347.

21. CONSTRUCTION JOINTS NOT SHOWN ON THE DRAWINGS MUST BE MADE AND LOCATED TO LEAST IMPAIR THE STRENGTH OF THE STRUCTURE.
A) NO HORIZONTAL CONSTRUCTION JOINTS WILL BE PERMITTED IN BEAMS, GIRDERS AND SLABS.
B) LOCATION OF ANY CONSTRUCTION JOINT NOT SHOWN IS SUBJECT TO REVIEW AND ACCEPTANCE BY ENGINEER.

22. INTERNAL VIBRATION, PROPERLY APPLIED IS THE REQUIRED METHOD OF CONSOLIDATING PLASTIC CONCRETE.

23. PROVIDE 3/4" CHAMFER ON ALL EXPOSED CORNERS OF COLUMNS, BEAMS AND WALLS UNLESS OTHERWISE NOTED ON ARCHITECTURAL DRAWINGS.

24. CONTRACTOR SHALL VERIFY LOCATIONS OF ALL OPENINGS, SLEEVES, AND SLAB RECESSES AS REQUIRED BY OTHER TRADES BEFORE CONCRETE IS PLACED. NO SLEEVE, OPENINGS, OR INSERT MAY BE PLACED IN BEAMS, JOISTS, OR COLUMN UNLESS APPROVED BY THE ENGINEER.

25. CONTRACTOR SHALL VERIFY EMBEDDED ITEMS INCLUDING, BUT NOT LIMITED TO, ANCHOR BOLTS, BOLT CLUSTERS, WELD PLATES, ETC., BEFORE PLACING CONCRETE. NOTIFY ENGINEER OF ANY CONFLICTS WITH REBAR.

26. SEE ARCHITECTURAL DRAWINGS AND SPECIFICATIONS FOR REQUIRED CONCRETE FINISHES, AND ADDITIONAL CONCRETE MIX DESIGN CRITERIA FOR POLISHED CONCRETE SURFACES.

27. SLOPE WALKWAYS TO DRAIN AWAY FROM THE BUILDING.

28. BUILDING FLOOR AND SITE SLABS-ON-GRADE SHALL BE 4" MINIMUM THICKNESS, UNLESS NOTED OTHERWISE.
A) REINFORCED WITH 6X6 - W1.4 X W1.4 W.W.F.
B) PLACED ON 10 MIL POLYETHYLENE VAPOR RETARDER. LAP 6" AND TAPE ALL JOINTS.
C) SAW-CUT CONTROL JOINTS @ LESS THAN OR EQUAL TO 16'-0" EACH WAY.
D) PROVIDE HOUSEKEEPING PADS AS REQUIRED.
E) SEE PLAN FOR APPARATUS BAY SLAB.
F) PROVIDE SUBGRADE MATERIALS AND COMPACT PER GEOTECH REQUIREMENTS.

29. TESTING
A) A QUALIFIED TESTING LAB SHALL BE RETAINED TO PERFORM QUALITY CONTROL WORK AND ON-SITE TESTING.
B) AIR TEST - ASTM C231
C) SLUMP TEST - ASTM 143
D) MOLD AND CURE TEST CYLINDERS (ASTM C-31) AND TEST CYLINDERS FOR STRENGTH (ASTM C39). TAKE ONE TEST - FOUR CYLINDERS FOR EACH DAYS POUR OF 50 CUBIC YARDS, OR FRACTION THEREOF. TEST ONE CYLINDER AT 7 DAYS, TWO AT 28 DAYS AND ONE HOLD. TEST CYLINDER SAMPLES SHALL BE TAKEN AT THE POINT OF DISCHARGE WHEN USING A PUMP.
E) ONE COPY OF ALL TEST REPORTS SHALL BE SENT DIRECTLY TO THE OWNER, ENGINEER, ARCHITECT AND GENERAL CONTRACTOR.

30. CONTRACTOR SHALL PROVIDE FLATNESS AND LEVELNESS IN CONCRETE SLABS PER ACI 302.1R, FIG. 8.15.1.1 MINIMUM REQUIRED "F" NUMBERS FOR TYPE OF SLAB USE. REFER TO ACI 117 FOR FLOOR TOLERANCES.

31. REPAIR ANY CRACKS OR DEFECTIVE AREAS THAT WILL RESTORE THE AFFECTED SURFACE OR AREAS TO THEIR FULL DESIGN STRENGTH AND APPEARANCE. CONTACT THE STRUCTURAL ENGINEER FOR ADVICE AND EVALUATION.

32. ACCEPTANCE OF THE STRUCTURE WILL BE MADE IN CONFORMANCE WITH ACI 301.

33. ALL CAST-IN-PLACE CONCRETE MUST BE MAINTAINED WITH MINIMAL MOISTURE LOSS AT A RELATIVELY CONSTANT TEMPERATURE FOR A MINIMUM OF 7 DAYS FOLLOWING THE PLACING OF THE CONCRETE BY THE USE OF A WATER SPRAY, WATER SATURATED FABRIC, MOISTURE RETAINING MEMBRANE OR LIQUID CURING COMPOUND.

30. CURE SLABS-ON-GRADE FOR THE FIRST 72 HOURS BY THE USE OF:
A) FOG SPRAYING
B) PONDING
C) SPRINKLING
D) CONTINUOUSLY WET ABSORPTIVE MATS OR FABRIC
E) CONTINUE CURING BY USE OF MOISTURE RETAINING COVER UNTIL CONCRETE HAS OBTAINED ITS SPECIFIED 28 DAY COMPRESSIVE STRENGTH
F) OR LIQUID CURING COMPOUND AFTER FINISHING PROCESS IS COMPLETED.
G) CONCRETE WET CURE TIME TO BE 7 DAYS MINIMUM AT 50 DEGREES MINIMUM TEMPERATURE.

35. SUBMIT MATERIALS AND METHOD OF CURING FOR REVIEW.

36. DO NOT USE MOISTURE RETAINING CURING COMPOUNDS FOR CURING SURFACES TO RECEIVE CARPET, FLEXIBLE FLOORING, CERAMIC TILED FLOORS OR OTHER SPECIFIED FLOOR SYSTEMS, UNLESS IT HAS BEEN DEMONSTRATED THAT SUCH COMPOUNDS WILL NOT PREVENT BOND.

37. DO NOT PERMIT CONCRETE NOT FULLY CURED TO BE EXPOSED TO EXCESSIVE TEMPERATURE CHANGES OR HIGH WINDS.

38. POUR ALL GROUND SLABS ON 10 MIL MINIMUM VAPOR RETARDER IN COMPLIANCE WITH ASTM E1745, LAPPED 6" MINIMUM AND FULLY TAPED.

PRECAST CONCRETE U-LINTELS AND SILLS

1. UNITS SHALL BE FABRICATED BY A FIRM ENGAGED IN THE MANUFACTURING OF PRECAST AND PRE-STRESSED CONCRETE U-LINTELS AND SILLS FOR A MINIMUM OF 5 YEARS. FABRICATOR SHALL HAVE A QUALITY ASSURANCE PROGRAM THAT COMPLIES WITH THE PROCEDURES OF MANUAL 116 BY THE PRECAST/PRE-STRESSED CONCRETE INSTITUTE PCI).

2. PLANT RECORDS OF PRODUCTION AND QUALITY CONTROL SHALL BE KEPT IN ACCORDANCE WITH PCI RECOMMENDATIONS AND MADE AVAILABLE UPON REQUEST FOR THE ARCHITECT/ENGINEER.

3. CODES AND STANDARDS:
A) AMERICAN SOCIETY FOR TESTING AND MATERIALS(ASTM)
1) C33 - SPECIFICATION FOR CONCRETE AGGREGATES
2) C150 - SPECIFICATION FOR PORTLAND CEMENT
B) PRECAST/PRE-STRESSED CONCRETE INSTITUTE (PCI) STANDARDS: MANUAL FOR QUALITY CONTROL FOR PRECAST AND PRE-STRESSED CONCRETE MNL-116.
C) AMERICAN CONCRETE INSTITUTE: BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE (ACI 318)
D) AMERICAN CONCRETE INSTITUTE: BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES (ACI 530)

4. CONCRETE MATERIALS:
A) PORTLAND CEMENT: ASTM C150 TYPE I OR III, GRAY COLOR
B) AGGREGATES: ASTM C33
C) WATER: POTABLE
D) ADMIXTURES: SHALL NOT CONTAIN CALCIUM CHLORIDE OR CHLORIDE IONS

5. REINFORCING
A) DEFORMED REINFORCEMENT: ASTM A615 GRADE 40 OR 60.
B) PRE-STRESSING STRAND: ASTM A416 270 KSI LL.

6. U-LINTEL UNITS 14 FEET IN OVERALL LENGTH AND SHORTER SHALL BE MADE OF CONCRETE WITH A MINIMUM STRENGTH OF 3500 PSI AT 28 DAYS.

7. SILL UNITS SHALL BE HI-DRI PRECAST SILLS MADE OF CONCRETE WITH A MINIMUM STRENGTH OF 3000 PSI AT 28 DAYS. SEE ARCHITECTURAL DRAWINGS FOR SPECIAL REQUIREMENTS.

8. UNITS SHALL BE SAND BLOCK FINISH EXCEPT PRE-STRESSED, 8" WIDE, AND 12" WIDE U-LINTELS SHALL BE SMOOTH FORM FINISHED.

9. PRECAST CONCRETE U-LINTELS SHALL BE DESIGNED BY A LICENSED DELEGATED ENGINEER.

10. SUBMITTALS
A) PROVIDE MANUFACTURER'S CATALOG ENGINEERING DATA.
B) MANUFACTURER SHALL RATE U-LINTEL UNITS FOR GRAVITY, UPLIFT, AND LATERAL LOADS IN UNITS OF POUNDS PER LINEAR FOOT.

PRECAST PRE-STRESSED HOLLOW CORE PLANKS

1. MANUFACTURER SHALL HAVE BEEN ENGAGED IN THE DESIGN AND PRODUCTION OF PRECAST PRESTRESSED COMPONENTS FOR AT LEAST FIVE (5) YEARS.

2. THE HOLLOW-CORE PRECAST SYSTEM AND ALL ACCESSORIES SHALL BE DESIGNED BY A LICENSED DELEGATED ENGINEER WHO SHALL PREPARE DESIGN CALCULATIONS AND SUPERVISE THE PREPARATION OF SHOP DRAWINGS.

3. WHERE CONCRETE TOPPING IS SHOWN, PRECAST MEMBERS SHALL BE DESIGNED FOR COMPOSITE ACTION TO SUPPORT SUPERIMPOSED LIVE LOADS AS GIVEN IN THE NOTES PLUS ALL DEAD LOADS AND THE WEIGHT OF PRECAST AND TOPPING. SEE PLAN FOR ANY CONCENTRATED LOADS.

4. WHERE TOPPING IS RECESSED OR NOT SPECIFIED, PRECAST MEMBERS SHALL BE DESIGNED FOR NON-COMPOSITE ACTION.

5. PRECAST MANUFACTURER SHALL BE A PARTICIPANT IN THE PCI PLANT CERTIFICATION PROGRAM AND SHALL BE A MEMBER OF PCI.

6. CONCRETE: 5000 PSI AT 28 DAYS. BATCH PLANT AND MIXING EQUIPMENT SHALL BE AT LEAST EQUAL TO THAT SPECIFIED BY ASTM C94, SPECIFICATION FOR READY MIX CONCRETE.

7. CODES AND STANDARDS:

ACI 318	BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE.
ASTM A416	STRESS-RELIEVED STRAND FOR PRESTRESSED CONCRETE
ASTM C33	CONCRETE AGGREGATES
ASTM C150	PORTLAND CEMENT
ASTM C330	LIGHTWEIGHT AGGREGATES FOR STRUCTURAL CONCRETE

8. MAINTAIN RECORDS OF CONCRETE COMPRESSIVE TESTS OF SPECIMENS REPRESENTATIVE OF THE WORK. AT LEAST THREE (3) CYLINDERS SHALL BE MADE FOR EACH DAY'S PRODUCTION AND TESTED AT 28 DAYS.

9. CONFORM TO REQUIREMENTS DEFINED IN PRESTRESSED CONCRETE INSTITUTE'S MANUAL 116 - "MANUAL FOR QUALITY CONTROL FOR PLANTS AND PRODUCTION OF PRECAST, PRESTRESSED CONCRETE PRODUCTS."

10. MANUFACTURER SHALL BE LISTED IN THE UNDERWRITERS FIRE RESISTANCE DIRECTORY OR SUBMIT/RATIONAL DESIGN OF STRUCTURAL MEMBERS FOR FIRE RESISTANCE AS PER MNL-124 BY THE "PRESTRESSED CONCRETE INSTITUTE".

11. FURNISH CERTIFICATION THAT PRESTRESS STRAND COMPLIES WITH SPECIFICATIONS WHEN REQUIRED.

12. MANUFACTURER SHALL NOTIFY ARCHITECTS AND GENERAL CONTRACTOR IN WRITING PRIOR TO CONSTRUCTION WHEN DESIGN PLANK CAMBER IS EXPECTED TO EXCEED THE PCI SAFE LOAD AND CAMBER TABLE VALUES.

13. CONCRETE MATERIALS:
A) PORTLAND CEMENT TYPE - ASTM C 150.
B) AGGREGATES - ASTM C330 OR C33.
C) CONCRETE SHALL BE ZERO (0) SLUMP.

14. FINE GROUT:
A) ONE PART PORTLAND CEMENT
B) THREE PARTS SAND
C) 6" SLUMP

15. THOROUGHLY CLEAN THEN GROUT ALL KEYWAYS.

16. CONCRETE TOPPING:
A) 3000 PSI COMPRESSIVE STRENGTH EXCEPT 5000 PSI AT EXTERIOR BALCONIES
B) 2" MINIMUM THICKNESS (WHEN DESIGNED FOR COMPOSITE ACTION)
C) 6X6 W1.4XW1.4 WWF REINFORCEMENT FURNISHED IN SHEETS AND INSTALLED ON CHAIRS OR RUNNERS AT MID-DEPTH OF TOPPING.

17. PRECAST SLAB SHALL BE PROPERLY CLEANED AND WETTED PRIOR TO TOPPING APPLICATION.

18. VERIFY THE EXACT SIZE AND LOCATION OF ALL FLOOR OPENINGS FOR MECHANICAL EQUIPMENT WITH THE MECHANICAL CONTRACTOR PRIOR TO FABRICATION OF MATERIALS.

19. NO OPENINGS MAY BE CUT OR DRILLED THROUGH THE PRECAST MEMBERS WITHOUT PRIOR APPROVAL FROM THE MANUFACTURER.

20. OPENINGS REQUIRING SUPPLEMENTARY STEEL HANGERS SHALL BE PROVIDED BY THE MANUFACTURER.

21. INSERTS, BRACES, STRONGBACKS, AND OTHER ACCESSORIES SHALL BE INSTALLED BY THE MANUFACTURER IN ACCORDANCE WITH RICHMOND BULLETIN NO. 8, "PRODUCTS FOR PRECAST/PRESTRESSED CONCRETE CONSTRUCTION". ALSO PROVIDE ANY SPECIAL REINFORCING TO PREVENT FLEXURAL CRACKS DURING SHIPPING AND INSTALLATION.

22. PRE-STRESSED CONCRETE SLABS SHALL BE FREE OF HONEYCOMBING AND CHAMFERS SHALL BE UNIFORM IN SIZE. UPON COMPLETION OF ERECTION, ERECTOR SHALL REPAIR CHIPPED MEMBERS.

23. PRE-STRESSED CONCRETE UNITS SHALL BE ERECTED INTO FINAL POSITION UNDER THE SUPERVISION OF AN EMPLOYEE OF THE MANUFACTURER, OR A COMPETENT ERECTOR.

24. PRESTRESSED CONCRETE UNITS SHALL BE ALIGNED AND LEVELED PRIOR TO GROUTING KEYWAY JOINTS.

25. ANY GAPS BETWEEN PLANK AND STRUCTURE ABOVE AND BELOW CREATED BY CAMBER OR UNEVEN SURFACES ARE TO BE PACKED SOLID WITH GROUT.

26. PRE-STRESSED CONCRETE UNITS BEARING REQUIREMENTS 3-1/2" FULL BEARING, 2" SIDE BEARING, UNLESS DETAILED OTHERWISE ON DRAWINGS.

27. PRE-STRESSED CONCRETE UNITS SHALL BEAR ON BEARING PADS AT END SUPPORTS AS REQUIRED BY PRECAST MANUFACTURER TO ENSURE PROPER BEARING PER PCI STANDARDS. BEARING PADS SHALL BE MULTIMONOMER PLASTIC BEARING STRIPS MANUFACTURED EXPRESSLY FOR BEARING PURPOSES.

28. IT IS THE GENERAL CONTRACTORS RESPONSIBILITY TO ENSURE A MINIMUM OF 36 HOURS OF CONCRETE CURING OF ALL LINTELS AND BEAMS PRIOR TO ERECTION OF HOLLOW CORE SLABS.

29. THE GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR PROVIDING SUITABLE ACCESS TO THE BUILDING, PROPER DRAINAGE AND FIRM/LEVEL SOIL BEARING FOR THE HAULING AND ERECTION EQUIPMENT TO OPERATE UNDER THEIR OWN POWER.

30. THE GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR:
A) PROVIDING TRUE AND LEVEL BEARING SURFACES ON ALL FIELD PLACED BEARING WALLS AND OTHER FIELD PLACED SUPPORTING MEMBERS.
B) ALL PIPES, STACKS, CONDUIT, AND OTHER SUCH ITEMS SHALL BE STUBBED OF AT A LEVEL LOWER THAN THE BEARING PLANE OF THE PRESTRESSED CONCRETE SLABS, UNTIL AFTER THE SLABS ARE SET IN PLACE.

31. SUBMITTALS:
A) SUBMIT SHOP DRAWINGS OF PRECAST CONCRETE COMPONENTS: ERECTION PLAN, CONNECTION DETAILS, DESIGN LOADS, AND ALL OPENINGS. SHOP DRAWINGS SHALL BE SIGNED AND SEALED BY THE DELEGATED ENGINEER.
B) SIGNED AND SEALED DESIGN CALCULATIONS, PREPARED BY THE DELEGATED ENGINEER, SHALL BE SUBMITTED WITH SHOP DRAWINGS.
C) CERTIFICATION LETTER FOR PCI COMPLIANCE SHALL BE SUBMITTED WITH SHOP DRAWINGS.

32. SUBMITTED SHOP DRAWINGS MUST BE CHECKED AND SIGNED BY THE GENERAL CONTRACTOR.

MASONRY

1. HOLLOW LOAD BEARING UNITS SHALL CONFORM TO ASTM C90, NORMAL WEIGHT, TYPE II. MINIMUM NET COMPRESSIVE UNIT STRENGTH = 2000 PSI. (NET AREA COMPRESSIVE MASONRY STRENGTH F'M = 1500 PSI).

2. MORTAR SHALL BE TYPE S AND CONFORM TO ASTM C270 (PROPORTION OR PROPERTY SPECIFICATION) WITH A MINIMUM AVERAGE COMPRESSIVE STRENGTH AT 28 DAYS OF 2500 PSI (TYPE M) AND 1800 PSI (TYPE S).

3. COARSE GROUT SHALL CONFORM TO ASTM C476:
A) 2500 PSI AT 28 DAYS.
B) 1/4" MAXIMUM AGGREGATE.
C) 8" - 11" SLUMP.

4. CODES AND STANDARDS:
A) SPECIFICATIONS FOR MASONRY STRUCTURES - ACI 530.1/ASCE 6/ TMS 602 IS INCLUDED BY REFERENCE IN ITS ENTIRETY.
B) BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES - ACI 530/ ASCE 5/TMS 402.

5. A REINFORCED CONCRETE TIE BEAM SHALL BE PROVIDED IN ALL WALLS SHOWN ON THE STRUCTURAL DRAWINGS AT EACH FLOOR AND THE ROOF. USE GALVANIZED MESH-TYPE CELL CAPS. PROVIDE CORNER BARS AT ALL BEAM CORNERS TO MATCH HORIZONTAL BARS.

6. UNLESS NOTED OTHERWISE, TIE BEAMS SHALL BE AS FOLLOWS:
A) FLOOR LEVEL: 8X16, 4000 PSI NW CONCRETE WITH (2) #5 TOP AND BOTTOM #3 STIR AT 24" ON CENTER.

B) ROOF LEVEL: 8" CMU: 8X16, 4000 PSI NW CONCRETE WITH (2) #5 TOP AND BOTTOM #3 STIR AT 24" ON CENTER.
12" CMU: 12X16, 4000 PSI NW CONCRETE WITH (2) #5 TOP AND BOTTOM #3 STIR AT 24" ON CENTER.

7. VERTICAL BARS SHALL BE HELD IN POSITION AT THE TOP AND BOTTOM OF BAR AND AT 8'_0" O.C. MAXIMUM WITH A MINIMUM CLEARANCE OF 1/2" FROM MASONRY. THE CLEAR DISTANCE BETWEEN BARS SHALL NOT BE LESS THAN ONE BAR DIAMETER, NOR LESS THAN 1". CENTER BARS IN WALLS U.N.O.

8. VERTICAL REINFORCING SHALL BE AS SHOWN ON THE DRAWINGS. FILL CELLS WITH COARSE GROUT AS SPECIFIED. PROVIDE ACI 90 DEGREE STANDARD HOOKS INTO FOOTING AND ROOF TIE BEAM. LAP SPLICE VERTICAL REINFORCEMENT ABOVE FOOTING AND ABOVE EACH FLOOR LEVEL UNLESS NOTED OTHERWISE. MAINTAIN VERTICAL REINFORCING SHOWN ON PLANS ABOVE AND BELOW MASONRY OPENINGS. CONTINUE FOUNDATION DOWELS BELOW ALL MASONRY OPENINGS.

9. REINFORCED FILL CELLS ARE TO BE CLEAN AND FREE OF ANY FOREIGN MATERIAL OR DEBRIS. REMOVE ANY INSULATING MATERIAL FROM CELLS, INCLUDING POLYSTYRENE INSULATING INSERTS, PRIOR TO GROUT POUR.

10. REINFORCING BARS SHALL BE STRAIGHT EXCEPT FOR BENDS AROUND CORNERS AND WHERE BENDS OR HOOKS ARE DETAILED ON THE PLANS.

11. REINFORCING BARS SHALL BE LAPPED PER MASONRY LAP SCHEDULE MINIMUM (UNLESS OTHERWISE NOTED) WHERE SPLICED AND SHALL BE WIRED TOGETHER.

12. WHEN A FOUNDATION DOWEL DOES NOT LINE UP WITH A VERTICAL CORE, IT SHALL NOT BE SLOPED MORE THAN ONE HORIZONTAL IN SIX VERTICALS. DOWELS SHALL BE GROUTED INTO A CORE IN VERTICAL ALIGNMENT, EVEN THOUGH IT IS IN AN ADJACENT CELL TO THE VERTICAL WALL REINFORCEMENT.

13. PROVIDE HORIZONTAL WALL REINFORCING (9 GA.) HOT DIPPED GALVANIZED LADDER TYPE DUR-O-WALL (OR EQUIVALENT) AT 16" O.C. JOINT REINFORCING SHALL CONFORM TO ASTM A-951.

14. PROVIDE HORIZONTAL JOINT REINFORCEMENT AT DOORS AND WINDOWS FOR FIRST AND SECOND BLOCK COURSE ABOVE AND BELOW APERTURES. RUN REINFORCING CONTINUOUS OR EXTEND TWO FEET FROM APERTURE EDGE.

15. WIRE REINFORCEMENT SHALL BE LAPPED AT LEAST 6" AT SPLICES AND SHALL CONTAIN AT LEAST ONE CROSS WIRE OF EACH PIECE OF REINFORCEMENT IN THE LAPPED DISTANCE.

16. CLEANOUTS SHALL BE PROVIDED IN THE BOTTOM COURSE OF MASONRY IN EACH GROUT POUR WHEN THE POUR HEIGHT EXCEEDS 5'. CLEANOUTS TO BE SAW-CUT 4" X 4".

17. GROUT POUR HEIGHT SHALL NOT EXCEED 12'. PLACE GROUT IN 6' MAX. LIFTS HEIGHTS.

18. CONSOLIDATE GROUT POURS AT THE TIME OF PLACEMENT BY MECHANICAL MEANS AND RECONSOLIDATE AFTER INITIAL WATER LOSS AND SETTLEMENT.

19. ALL MASONRY FOUNDATION STEMWALLS SHALL BE FULLY GROUTED.

20. STORE BLOCKS ON PALLETS AND COVER WITH PLASTIC SHEETING.

21. PLACE MASONRY IN RUNNING BOND WITH 3/8" MORTAR JOINTS. PROVIDE COMPLETE COVERAGE FACE SHELL MORTAR BEDDING, HORIZONTAL AND VERTICAL. FULLY MORTAR WEBS IN ALL COURSES OF PIERS, COLUMNS, AND PILASTERS AND ADJACENT TO GROUTED CELLS.

22. SEE DRAWINGS FOR MASONRY CONTROL JOINT LOCATIONS.

23. SUBMITTALS:
A) SUBMIT PROPOSED GROUT MIX DESIGN PRIOR TO CONSTRUCTION.
B) SUBMIT PROPOSED MORTAR MIX DESIGN PRIOR TO CONSTRUCTION.
C) SUBMIT DETAILED SHOP DRAWINGS OF REINFORCING BARS SHOWING NUMBER, SIZE, AND LOCATION. INCLUDE BAR LISTS AND BEND DIAGRAMS. INCLUDE ALL REQUIRED DIMENSIONS AND ELEVATIONS.
D) SUBMIT COMPRESSIVE STRENGTH TESTS OF PROPOSED MASONRY UNITS PRIOR TO CONSTRUCTION. MASONRY UNITS ARE TO BE TESTED IN ACCORDANCE WITH ASTM C140.

24. A QUALIFIED TESTING LABORATORY SHALL BE RETAINED BY THE CONTRACTOR TO PERFORM THE FOLLOWING TESTS:
A) SAMPLE AND TEST GROUT IN ACCORDANCE WITH ASTM C1019 FOR EACH 5000 SQ. FT. OF MASONRY.
B) SLUMP TESTS - ASTM C143.
C) MASONRY PRISM TEST IN ACCORDANCE WITH ASTM C1314. PROVIDE ONE SET OF 3 PRISMS PRIOR TO CONSTRUCTION AND DURING CONSTRUCTION FOR EACH 5000 SQ. FT. OF WALL.

25. PROVIDE 8" DEEP PRECAST REINFORCED CONCRETE LINTELS OVER ALL MASONRY OPENINGS NOT SHOWN TO HAVE A STRUCTURAL BEAM. MINIMUM END BEARING = 8". LINTEL WIDTH TO MATCH MASONRY WIDTH.

26. TOPS OF PARTIALLY CONSTRUCTED WALLS SHALL BE COVERED WITH VISQUEEN WHENEVER RAIN OCCURS AND AT THE END OF THE WORK DAY.

DRILL-IN BOLTS, SCREWS AND DOWELS

1. EPOXY BOLTS SHALL BE ASTM A307 THREADED BOLTS WITH NUTS AND WASHERS INSTALLED IN ACCORDANCE WITH MANUFACTURER'S INSTRUCTIONS. PROVIDE "EPOXY-TIE SET" ANCHORS BY SIMPSON STRONG-TIE OR EQUAL.

2. ANCHORING ADHESIVE SHALL BE A TWO-COMPONENT SOLID EPOXY-BASED SYSTEM SUPPLIED IN MANUFACTURER'S STANDARD SIDE-BY-SIDE CARTRIDGE AND DISPENSED THROUGH A STATIC-MIXING NOZZLE SUPPLIED BY THE MANUFACTURER. EPOXY SHALL MEET THE MINIMUM REQUIREMENTS OF ASTM C-881 SPECIFICATION FOR TYPE I, II, IV AND V, GRADE 3, CLASS B, C AND D AND MUST DEVELOP A MINIMUM 12,650 PSI COMPRESSIVE YIELD STRENGTH AFTER 7-DAY CURE.

3. EXPANSION BOLTS SHALL BE "WEDGE-ALL" ANCHORS BY SIMPSON STRONG-TIE OR EQUAL.

4. EXPANSION BOLTS SHALL BE NON-BOTTOM BEARING, WEDGE-STYLE EXPANSION ANCHOR FOR USE IN SOLID CONCRETE OR GROUT FILLED MASONRY WITH A ONE-PIECE NON-CORROSIVE CLIP FOR UNIFORM HOLDING CAPACITY THAT INCREASES AS TENSION IS APPLIED. INSTALL IN ACCORDANCE WITH MANUFACTURER'S INSTRUCTIONS.

5. MASONRY SCREWS SHALL BE 1/4" DIAMETER WITH 1-1/2" MINIMUM EMBEDMENT INSTALLED IN DRILLED HOLES USING AN APPROPRIATE BIT DIAMETER.

6. SCREWS SHALL BE MADE FROM TEN HUNDRED SERIES COLD-ROLLED STEEL AND SHALL BE FLUOROCARBON COATED. PROVIDE "TITEN" SCREWS BY SIMPSON STRONG-TIE OR EQUAL.

7. DRILL-IN REBAR DOWELS SHALL BE SET USING A TWO-PART EPOXY AS DESCRIBED ABOVE.

8. NAILS, SCREWS, BOLTS AND/OR OTHER FASTENERS OF ANY KIND AND DESCRIPTION SHALL BE OF DOMESTIC (USA) MANUFACTURER; NO FOREIGN MADE FASTENERS, NOR FASTENERS MADE OF FOREIGN MADE MATERIALS WILL BE ALLOWED 0N THE PROJECT.

McCarthy and Associates, Inc.
CONSULTING ENGINEERS
www.mccarthyassoc.com
601 N. CONGRESS AVE. SUITE 106A
Delray Beach, FL. 33445
(561)265-6864
Florida Coa 4187
Robert J. Selinsky, P.E.
Florida P.E. 49952
McCarthy Project No. 11261

CURRIE
SOWARDS
AGUILA
ARCHITECTS

Architects, Planners & Interior Designers

AA26001584
134 N.E. 1st Avenue
Delray Beach, Florida 33444
TEL: 561 276-4951
FAX: 561 243-8184
E-mail: Office@CSA-Architects.com

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SEAL

PROJECT TITLE

CITY OF POMPANO BEACH FIRE STATION 103

3721 N.E. 12th AVE.
POMPANO BEACH, FL 33062

REVISIONS

THESE DRAWINGS ARE PREPARED PER ESTABLISHED INDUSTRY STANDARDS AND REPRESENT THE ARCHITECT AND ENGINEERS DESIGN CONCEPT. THEY ARE NOT INTENDED TO PROVIDE EVERY DETAIL OR CONDITION REQUIRED TO CONSTRUCT THE BUILDING. THE CONTRACTOR THROUGH SUBMITTALS AND OTHER COORDINATION EFFORTS IS FULLY RESPONSIBLE FOR PROVIDING A COMPLETE AND OPERATIONAL BUILDING WHETHER INDICATED ON THE PLANS OR NOT.

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STRUCTURAL SPECIFICATIONS

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STRUCTURAL GENERAL NOTES CONT.

STRUCTURAL STEEL

1. STRUCTURAL STEEL SHALL CONFORM TO THE AISC "SPECIFICATION FOR BUILDINGS", LATEST EDITION, EXCEPT CHAPTER 4.2.1 OF THE CODE OF STANDARD PRACTICE.

2. WELDED CONNECTIONS SHALL CONFORM TO THE LATEST REVISED CODE OF THE AMERICAN WELDING SOCIETY, AWS D1.1. ALL WELDING SHALL BE PERFORMED USING E70XX, LOW HYDROGEN ELECTRODES. ELECTRODES ARE TO BE PROTECTED FROM MOISTURE.

3. CONNECTIONS TO BE DOUBLE ANGLE FRAMED BEAM CONNECTION PER AISC UNLESS NOTED OTHERWISE. ALL BOLTS TO BE 3/4" DIAMETER UNLESS NOTED OTHERWISE. SHOP CONNECTIONS MAY BE WELDED OR BOLTED. WELDS ARE TO BE EQUAL IN STRENGTH TO BOLTS. ALL FIELD CONNECTIONS ARE TO BE BOLTED WITH ASTM A325N OR A490 BOLTS (BEARING TYPE BOLTS WITH THREADS IN THE SHEAR PLANE) INCLUDING SUITABLE NUTS AND PLAIN HARDENED WASHERS. ALL BOLTS SHALL BE TIGHTENED SNUG TIGHT UNLESS OTHERWISE NOTED. DESIGN CONNECTIONS FOR THE LARGER OF EITHER THE SHEAR SHOWN ON THE DRAWINGS, (INDICATED AS "V =K" AT ENDS OF MEMBER) OR 55% OF THE MAXIMUM LOAD(IN KIPS) LISTED IN THE TABLES FOR "MINIMUM TOTAL FACTORED UNIFORM LOADS IN KIPS FOR BRACED, SIMPLE SPAN BEAMS BENT ABOUT THE STRONG AXIS" OF THE LATEST EDITION OF THE AISC "MANUAL OF STEEL CONSTRUCTION".

4. SIZE AND USE OF HOLES, SEE AISC TABLE J3.3.
A) LARGER HOLES ARE PERMITTED IN STANDARD COLUMN BASE PLATES. MAXIMUM HOLE DIAMETER = BOLT DIAMETER + 3/8". HARDENED WASHERS, TO COVER THE LARGER HOLE, SHALL BE PROVIDED.
B) SLOTTED HOLES: A PLATE WASHERS OR A CONTINUOUS BAR WITH STANDARD HOLES HAVING A SIZE SUFFICIENT TO COMPLETELY COVER THE SLOT AFTER INSTALLATION, AND A MIN. OF 5/16" THICK SHALL BE PROVIDED. TACK WELD NUT TO BOLT AFTER ERECTION.

5. STEEL BEAMS SHALL BE FABRICATED WITH THE NATURAL CAMBER (WITHIN THE MILL TOLERANCE) LOCATED ABOVE THE HORIZONTAL CENTERLINE BETWEEN THE END CONNECTIONS. ALL EXPOSED STEEL SHALL BE HOT DIPPED GALVANIZED.

6. VERIFY THE EXACT SIZE AND LOCATION OF ALL FLOOR AND ROOF OPENINGS FOR MECHANICAL EQUIPMENT WITH THE MECHANICAL CONTRACTOR PRIOR TO FABRICATION OF MATERIALS.

7. SHOP PRIME STEEL SURFACES EXCEPT THE FOLLOWING:
A) SURFACES EMBEDDED IN CONCRETE OR MORTAR. EXTEND PRIMING OF PARTIALLY EMBEDDED MEMBERS TO A DEPTH OF 2 INCHES.
B) SURFACES TO BE FIELD WELDED.
C) SURFACES TO BE HIGH-STRENGTH BOLTED WITH SLIP-CRITICAL CONNECTIONS.
D) SURFACES TO RECEIVE SPRAYED FIRE-RESISTIVE MATERIALS.
E) GALVANIZED SURFACES.

8. SURFACE PREPARATION: CLEAN SURFACES TO BE PAINTED. REMOVE LOOSE RUST AND MILL SCALE AND SPATTER, SLAG, OR FLUX DEPOSITS. PREPARE SURFACES ACCORDING TO THE FOLLOWING SPECIFICATIONS AND STANDARDS.

9. PRIMING: IMMEDIATELY AFTER SURFACE PREPARATION, APPLY PRIMER ACCORDING TO MANUFACTURER'S WRITTEN INSTRUCTIONS AND AT RATE RECOMMENDED BY SSPC TO PROVIDE A DRY FILM THICKNESS OF NOT LESS THAN 1.5 MILS. USE PRIMING METHODS THAT RESULT IN FULL COVERAGE OF JOINTS, CORNERS, EDGES, AND EXPOSED SURFACES.
A) STRIPE PAINT CORNERS, CREVICES, BOLTS, WELDS, AND SHARP EDGES.
B) APPLY TWO COATS OF SHOP PAINT TO INACCESSIBLE SURFACES AFTER ASSEMBLY OR ERECTION. CHANGE COLOR OF SECOND COAT TO DISTINGUISH IT FROM FIRST.

10. PRIME AND PAINT ALL FIELD WELDS AFTER INSPECTION.

11. A QUALIFIED TESTING LABORATORY SHALL BE RETAINED BY THE CONTRACTOR TO PERFORM THE FOLLOWING TESTS.
A) VISUALLY INSPECT ALL STEEL MEMBERS AND CONNECTIONS.

12. ONE COPY OF ALL TEST REPORTS SHALL BE SENT DIRECTLY TO OWNER, ARCHITECT, STRUCTURAL ENGINEER, AND GENERAL CONTRACTOR.

13. STEEL SHALL CONFORM TO:
WIDE FLANGE (WF)(WT)----------ASTM A992 (50 KSI)
SHAPES (L, C, MC)----------ASTM A36
HOLLOW STRUCTURAL SECTIONS (HSS)--ASTM A500 (46 KSI)(TYPE B)
ANCHOR BOLTS----------ASTM A307
FRAMING BOLTS----------ASTM A325N OR A490N
SHEAR STUDS----------ASTM A108
WELDING ELECTRODES----------E70XX

14. FASTENERS AND MATERIALS USED FOR WELDING OR OTHERWISE SECURING COMPONENTS ONE TO ANOTHER SHALL BE OF DOMESTIC (USA MADE) MANUFACTURE. SIMILARLY, ALL MATERIALS USED IN THE MANUFACTURING PROCESS SHALL BE FROM A DOMESTIC SOURCE.

15. OPENINGS THROUGH STEEL BEAMS SHALL BE PROVIDED AS DETAILED ON THE DRAWINGS. ALL SUCH OPENINGS SHALL BE MACHINE CUT IN THE SHOP. ALL RECTANGULAR OPENINGS SHALL HAVE A CORNER RADIUS OF 2 TIMES THE WEB THICKNESS, 1/2" MINIMUM.

16. SHOP AND FIELD WELDS SHALL BE DONE BY A.W.S. CERTIFIED WELDERS. PROVIDE CURRENT CERTIFICATES UPON REQUEST.

17. NO SPLICES SHALL BE PERMITTED IN ANY STRUCTURAL STEEL MEMBER UNLESS SHOWN ON APPROVED SHOP DRAWINGS.

18. STEEL STAIRS AND/OR LADDERS SHALL BE DESIGNED FOR 100 PSF LIVE LOAD BY A LICENSED DELEGATED ENGINEER, WHO SHALL SUBMIT SIGNED AND SEALED SHOP DRAWINGS. SHOP DRAWINGS SHALL SPECIFY ALL DESIGN LOADS.

19. SUBMITTALS: CONTRACTOR SHALL SUBMIT DETAILED SHOP DRAWINGS SHOWING ALL STRUCTURAL STEEL LAYOUTS AND DETAILS, SIZES OF MEMBERS, TYPE OF STEEL, CONNECTION DETAILS, WELDS, BOLTS, ETC., AS REQUIRED TO FABRICATE AND ERECT ALL STRUCTURAL STEEL FRAMING. ALL CONNECTIONS NOT SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE BY THE DETAILER AND SUBMITTED ON SHOP DRAWINGS, SIGNED AND SEALED BY A REGISTERED FLORIDA DELEGATED ENGINEER.

20. NON-SHRINK GROUT SHALL BE: NONMETALLIC SHRINKAGE-RESISTANT GROUT, PREMIXED, NON-CORROSIVE, NON-STAINING PRODUCT CONTAINING SELECTED SILICA SANDS, PORTLAND CEMENT, SHRINKAGE COMPENSATING AGENTS, PLASTICIZING AND WATER-REDUCING AGENTS, COMPLYING WITH CRD-C621, CORPS OF ENGINEERS.

21. IF NOT SPECIFIED ON THE DRAWINGS, THE THROAT SIZE OF ANY FILLET WELD SHALL BE EQUAL TO 1/16" LESS THAN THE THINNEST CONNECTION COMPONENT.

22. NO FIELD WELDING OF GALVANIZED MEMBERS IS PERMITTED.

23. MINIMUM EMBEDMENT DEPTH OF ANCHOR BOLTS:
A) BEAMS, COLUMNS, WALLS = 6"
B) FOOTINGS = 3" FROM BOTTOM

24. ERECTION
A) BEFORE ERECTION, THE CONTRACTOR IS TO REMOVE ALL MUD, DIRT OR OTHER FOREIGN MATTER, WHICH ACCUMULATES DURING HANDLING AND STORAGE.
B) DRIFTING TO ENLARGE UNFAIR HOLES WILL NOT BE PERMITTED. DRILL SUCH HOLES TO ACCOMMODATE THE NEXT LARGER SIZE FASTENER, WHERE POSSIBLE.
C) AFTER ERECTION, CLEAN FIELD WELDS, BOLTED CONNECTIONS, AND ABRADED AREAS WHERE SHOP COAT HAS BEEN DAMAGED. SPOT AND PRIME AREAS USING SAME MATERIAL AS SHOP COAT.
D) SET ALL MEMBERS SO THAT, IN THEIR FINAL LOCATION, LEVEL, PLUMBNESS AND ALIGNMENT ARE WITHIN THE TOLERANCES PRESCRIBED BY AISC CODE.
E) DOUBLE CONNECTIONS THROUGH COLUMN WEBS OR AT BEAMS THAT FRAME OVER THE TOPS OF COLUMNS MUST BE DESIGNED TO HAVE AT LEAST ONE INSTALLED BOLT REMAIN IN PLACE TO SUPPORT THE FIRST BEAM WHILE THE SECOND BEAM IS BEING ERECTED. ALTERNATIVELY, THE FABRICATOR MUST SUPPLY A SEAT OR EQUIVALENT DEVICE WITH A MEANS OF POSITIVE ATTACHMENT TO SUPPORT THE FIRST BEAM WHILE THE SECOND BEAM IS BEING ERECTED.

OPEN WEB STEEL JOISTS AND JOIST GIRDERS (NOTED "JOISTS" HEREIN)

1. STEEL JOIST MANUFACTURER SHALL BE A MEMBER OF THE STEEL JOIST INSTITUTE.

2. STEEL JOISTS SHALL CONFORM TO THE REQUIREMENTS OF THE LATEST EDITION OF THE AISC STANDARD SPECIFICATIONS FOR OPEN WEB STEEL JOISTS K, LH, OR DLH SERIES (SEE PLAN), AND OSHA STEEL ERECTION STANDARD.

3. JOISTS, GIRDERS, AND ALL ACCESSORIES SHALL BE DESIGNED BY A LICENSED DELEGATED ENGINEER WHO SHALL PREPARE DESIGN CALCULATIONS AND SUPERVISE THE PREPARATION OF SHOP DRAWINGS.

4. VERIFY THE EXACT LOCATION AND WEIGHT OF ALL MECHANICAL EQUIPMENT WITH THE MECHANICAL CONTRACTOR PRIOR TO FABRICATION OF JOISTS.

5. ALL HANGERS TO SUPPORT MECHANICAL EQUIPMENT, ETC., TO BE SUPPORTED BY THE TOP OR BOTTOM CHORD OF JOISTS SHALL BE LOCATED AT THE PANEL POINT OF THE JOIST. IF HANGERS MUST BE LOCATED IN BETWEEN PANEL POINTS, PROVIDE JOIST STIFFENER AS INDICATED IN DETAILS. ALL HANGERS TO BE LOCATED AT THE CENTERLINE OF THE BOTTOM CHORD MEMBER.

6. NO MODIFICATION THAT AFFECTS THE STRENGTH OF A STEEL JOIST SHALL BE MADE WITHOUT THE APPROVAL OF THE DELEGATED ENGINEER.

7. DELEGATED ENGINEER SHALL DESIGN JOISTS AND BRIDGING FOR GRAVITY LOADS AND WIND NET UPLIFT: NET UPLIFT= 78 PSF
ROOF DL= 25 PSF
ROOF LL= 30 PSF

8. JOIST BRIDGING SHALL BE FURNISHED AND INSTALLED TO MEET THE DESIGN AND SPACING REQUIREMENTS OF THE SJI STANDARD SPECIFICATIONS. ALL BRIDGING AND BRIDGING ANCHORS SHALL BE COMPLETELY INSTALLED BEFORE CONSTRUCTION LOADS ARE PLACED ON THE JOISTS.

9. WHERE COLUMNS ARE NOT FRAMED IN AT LEAST TWO DIRECTIONS WITH SOLID WEB BEAMS, A STEEL JOIST SHALL BE FIELD-BOLTED AT THE COLUMN TO PROVIDE LATERAL STABILITY DURING ERECTION.

10. A 6"X6" MINIMUM VERTICAL STABILIZER PLATE TO RECEIVE THE JOIST BOTTOM CHORD MUST BE PROVIDED AT COLUMNS. THE STABILIZER PLATE MUST HAVE A 13/16 INCH HOLE FOR THE ATTACHMENT OF GUYING OR PLUMBING CABLES.

11. JOISTS AND ACCESSORIES SHALL HAVE ONE SHOP COAT OF PAINT MEETING THE MINIMUM PERFORMANCE REQUIREMENTS OF THE LATEST SJI SPECIFICATIONS. SEE ARCHITECT FOR PREFERRED COLOR.

12. SEE PLAN FOR ANY CONCENTRATED LOADS OR UNUSUAL CONDITIONS. ALL JOISTS SUBJECT TO SPECIAL LOADS OR CONDITIONS SHALL BE CONSIDERED "SPECIAL JOISTS", (SP).

OPEN WEB STEEL JOISTS AND JOIST GIRDERS (NOTED "JOISTS" HEREIN) CONT.

13. CONTRACTOR SHALL SUBMIT DETAILED SHOP DRAWINGS SHOWING JOISTS, BRIDGING, AND ALL CONNECTIONS. SHOP DRAWINGS SHALL BE SIGNED AND SEALED BY THE DELEGATED ENGINEER.

14. THE DELEGATED ENGINEER MUST BE PROVIDED WITH A COPY OF THESE DRAWINGS AND SPECIFICATIONS.

15. PROVIDE ONE ROW UPLIFT BRIDGING AT THE FIRST PANEL POINT FROM EACH SUPPORT.

16. A ROW OF BOLTED DIAGONAL BRIDGING MUST BE PROVIDED NEAR MIDSPAN OF ALL JOISTS SHOWN IN SJI TABLES A & B. DO NOT RELEASE HOISTING CABLES UNTIL THIS BRIDGING IS INSTALLED.

17. DURING THE CONSTRUCTION PERIOD, THE CONTRACTOR SHALL PROVIDE MEANS FOR ADEQUATE DISTRIBUTION OF CONCENTRATED LOADS SO THAT THE CARRYING CAPACITY OF ANY JOIST IS NOT EXCEEDED.

18. ONE END OF ALL JOISTS SHALL BE ATTACHED TO ITS SUPPORT IN ACCORDANCE WITH SJI SPECIFICATIONS BEFORE ALLOWING THE WEIGHT OF AN ERECTOR ON THE JOISTS.

19. SEE STANDARD JOIST SPECIFICATIONS FOR CAMBER REQUIREMENTS.

20. SUBMIT COMPLETE SHOP DRAWINGS FOR ALL JOISTS AND ACCESSORIES, AND A LETTER, SIGNED AND SEALED BY THE DELEGATED ENGINEER, CONFIRMING COMPLIANCE WITH THE DESIGN CRITERIA AND ALL APPLICABLE CODES.

METAL DECKING

1. METAL DECK WORK SHALL CONFORM TO THE REQUIREMENTS OF THE STEEL DECK INSTITUTE.

2. SLOPED METAL ROOF DECK SHALL BE 1 1/2" DEEP, 18 GA., WIDE RIB TYPE B AND G60 GALVANIZED. (VULCRAFT 1.5B18 OR EQUIVALENT.)

3. FASTEN ROOF DECK WITH (7) #10 TEK STITCH SCREWS AT EACH SUPPORT PER 36" WIDTH AND (6) #10 TEK SCREWS AT MIDSPAN OF SIDELAPS AS INDICATED ON PLANS.

4. FLAT ROOF METAL DECK SHALL BE 1 1/2" DEEP, 22 GA., GALVANIZED G90 TYPE "B" VENTED DECK. FASTEN W/ 36/7 WELD & (7) #10 TEK STITCH SCREWS.

5. MINIMUM FASTENING AT BUILDING PERIMETER OF DECK SHALL BE 5/8" DIAMETER PUDDLE WELDS AT 6" O.C.

6. METAL DECK AND SHEET METAL COATING DESIGNATION:

A. WITH STRUCTURAL CONCRETE OR INSULATING CONCRETE TOPPING - G90
B. WITHOUT STRUCTURAL CONCRETE OR INSULATING CONCRETE TOPPING - G60

7. INSTALL ALL DECKING 3 SPAN CONTINUOUS.

8. USE WELD WASHERS FOR ALL DECKING 24 GA. AND THINNER.

9. DO NOT HANG OR ATTACH DUCTWORK, CONDUIT, PIPING, EQUIPMENT, CEILINGS, ETC. FROM METAL DECKING.

10. ROOF DECK OPENINGS 12" DIAMETER OR LARGER ARE TO HAVE SUPPORT ANGLES PER TYPICAL DECK OPENING DETAIL, INCLUDING OPENINGS FOR ROOF SUMP PANS.

11. PRIME AND PAINT ALL FIELD WELDS AFTER INSPECTION WITH A GALVANIZED TOUCH-UP PAINT. (SEE NOTE BELOW)

12. SUBMITTALS: CONTRACTOR SHALL SUBMIT DETAILED SHOP DRAWINGS SHOWING LAYOUT OF DECK, TYPE OF DECK, ALL CONNECTIONS INCLUDING END WELDS, SEAM WELDS, INTERMEDIATE WELDS, AND ALL ACCESSORY MATERIAL SUCH AS CLOSURES, SUMPS FOR DRAINS, ETC.

13. A QUALIFIED TESTING LABORATORY SHALL BE RETAINED BY THE CONTRACTOR TO VISUALLY INSPECT ALL DECK WELDS AND FASTENERS.

COLD-FORM STEEL FRAMING

1. STEEL FRAMING SHALL CONFORM TO THE A.I.S.I. "SPECIFICATION FOR THE DESIGN OF COLD_FORMED STEEL STRUCTURAL MEMBERS".

2. COLD-FORMED STEEL FRAMING SYSTEM, INCLUDING STUDS, TRUSSES, CONNECTIONS, AND ALL ACCESSORIES, SHALL BE DESIGNED BY A DELEGATED ENGINEER WHO SHALL PREPARE CALCULATIONS AND SUPERVISE THE PREPARATION OF SHOP DRAWINGS.

3. WELDED CONNECTIONS SHALL CONFORM TO "CODE FOR WELDING IN BUILDING CONSTRUCTION, AWS D1.3".

4. ASTM A_568 STANDARD SPECIFICATION FOR GENERAL REQUIREMENTS FOR STEEL, CARBON AND HIGH STRENGTH LOW_ALLOY HOT ROLLED SHEET AND COLD ROLLED SHEET.

5. ALL STEEL FRAMING SHALL BE INSTALLED BY PERSONNEL EXPERIENCED IN LIGHT GAUGE STEEL FRAMING INSTALLATION.

6. WHERE STEEL FRAMING MEMBERS ARE COMPONENTS OF ASSEMBLIES INDICATED FOR A FIRE_RESISTANCE RATING, INCLUDING THOSE REQUIRED FOR COMPLIANCE WITH GOVERNING REGULATIONS, PROVIDE MEMBERS WHICH HAVE BEEN APPROVED BY GOVERNING AUTHORITIES HAVING JURISDICTION.

7. PROTECT LIGHT GAUGE STEEL FRAMING MEMBERS FROM RUSTING AND DAMAGE. DELIVER TO PROJECT SITE IN BUNDLES, FULLY IDENTIFIED WITH NAME, BRAND, TYPE AND GRADE. STORE OFF GROUND IN A DRY VENTILATED SPACE OR PROTECT WITH SUITABLE WATERPROOF COVERINGS.

8. WITH EACH TYPE OF STEEL FRAMING REQUIRED, PROVIDE MANUFACTURER'S STANDARD STEEL RUNNERS (TRACKS), BLOCKING, LINTELS, CLIP ANGLES, BRACING, REINFORCEMENTS, FASTENERS, AND ACCESSORIES AS RECOMMENDED BY MANUFACTURER FOR APPLICATIONS INDICATED, AS NEEDED TO PROVIDE A COMPLETE STEEL FRAMING SYSTEM.

9. FABRICATE METAL FRAMING COMPONENTS OF STRUCTURAL QUALITY SHEET STEEL WITH A MINIMUM YIELD POINT OF 50,000 PSI FOR STUDS, AND 33,000 PSI FOR RUNNERS; ASTM A653.

10. PROVIDE GALVANIZED FINISH TO METAL FRAMING COMPONENTS COMPLYING WITH ASTM A525 WITH A G60 COATING.

11. PROVIDE MANUFACTURER'S STANDARD STRUCTURAL "CEE" SHAPED STEEL STUDS OF SIZE, SHAPE, AND GAUGE INDICATED, WITH A NOMINAL 1_5/8" FLANGE AND MINIMUM 1/2" FLANGE RETURN LIP BY DIETRICH INDUSTRIES, INC. OR PRIOR APPROVED EQUAL.

12. INSTALL METAL FRAMING SYSTEMS IN ACCORDANCE WITH REVIEWED SHOP DRAWINGS.

13. WHERE REQUIRED, TEMPORARY BRACING SHALL BE PROVIDED UNTIL ERECTION IS COMPLETED.

14. RESISTANCE TO BENDING AND ROTATION ABOUT THE MINOR AXIS SHALL BE PROVIDED BY MECHANICAL LATERAL BRACING WHERE REQUIRED.

15. ATTACHMENTS OF SIMILAR COMPONENTS SHALL BE DONE BY WELDING, SCREW ATTACHMENT, OR BOLTING. WIRE TYING OF FRAMING COMPONENTS SHALL NOT BE PERMITTED.

16. WELDING OF MEMBERS LIGHTER THAN 18 GAUGE SHALL NOT BE PERMITTED.

17. SPLICES SHALL NOT BE PERMITTED.

18. FULLY INSTALL ALL BRIDGING BEFORE APPLYING LOADS.

19. STEEL ROOF TRUSSES:
A) TRUSS ERECTOR IS RESPONSIBLE FOR ALL TEMPORARY BRIDGING OF THE TRUSS SYSTEM DURING CONSTRUCTION.
B) TRUSSES SHALL BE DESIGNED SO THAT NO HORIZONTAL REACTIONS ARE IMPOSED ON THE SUPPORTING STRUCTURE UNDER VERTICAL LOAD.
C) PREFABRICATED TRUSSES AND PANELS SHALL BE SQUARE AND BRACED AGAINST RACKING.
D) TRUSS MANUFACTURER SHALL PROVIDE A BENT PLATE 3" X 3" X 14 GAGE TYPICAL AT ALL RIDGE AND VALLEY LINES. PROVIDE CONTINUOUS DECK SUPPORT BETWEEN TRUSSES AT ALL HIPS, RIDGES, VALLEYS, AND CHANGES IN ROOF SLOPE.

20. CONTRACTOR TO SUBMIT THE FOLLOWING:
A) SUBMIT COMPLETE STRUCTURAL SHOP DRAWINGS AND CALCULATIONS FOR THE ROOF TRUSS FRAMING SYSTEM. CALCULATIONS SHALL COVER ALL TRUSSES, BRACING, ATTACHMENT OF LIGHT GAUGE FRAMING TO LIGHT GAUGE FRAMING, AND ATTACHMENT OF LIGHT GAUGE FRAMING TO CONCRETE OR STRUCTURAL STEEL.
B) SUBMIT DETAILED SHOP DRAWINGS FOR STEEL TRUSSES SHOWING THE TYPE AND SPACING OF ALL MEMBERS. ALL ATTACHMENTS SHALL BE CLEARLY DETAILED ON THE DRAWINGS. INDICATED SUPPLEMENTAL STRAPPING, BRACING, CLIPS, AND OTHER ACCESSORIES REQUIRED FOR PROPER INSTALLATION.
C) SUBMIT CERTIFICATION OF MATERIALS FROM THE MANUFACTURER TO SHOW COMPLIANCE WITH THESE SPECIFICATIONS AND RELATED DRAWINGS.

21. SUBMITTALS SHALL BEAR THE SEAL OF THE DELEGATED ENGINEER.

22. SUBMITTED SHOP DRAWINGS MUST BE CHECKED AND SIGNED BY THE GENERAL CONTRACTOR.

McCarthy and Associates, Inc.
CONSULTING ENGINEERS
www.mccarthyassoc.com
601 N. CONGRESS AVE. SUITE 106A
Delray Beach, FL 33445
(561)265-6864
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Robert J. Selinsky, P.E.
Florida P.E. 49952
McCarthy Project No. 11261

THIS BUILDING IS DESIGNED AS AN ENCLOSED STRUCTURE. ALL EXTERIOR COMPONENTS (DOORS, WINDOWS, ETC.) MUST BE DESIGNED TO WITHSTAND THE WIND LOADINGS SPECIFIED FOR THE DESIGN OF COMPONENTS AND CLADDING IN THE TABLES. IN ADDITION, ALL AREAS OF EXTERIOR GLAZING MUST BE CERTIFIED FOR MISSILE IMPACT OR PROTECTED BY WIND-BORNE DEBRIS BY A SCREEN BARRIER.

GROSS WIND LOADS
MAIN ROOF
ROOFING MATERIALS

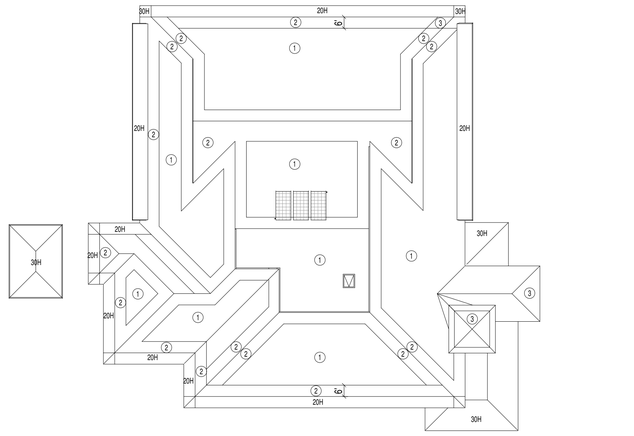
COMPONENTS AND CLADDING	ROOF ZONE			OVERHANG	
	1	2	3	2OH	3OH
PRESSURE (psf)	41.7	41.7	41.7	41.7	41.7
SUCTION (psf)	-66.3	-115.4	-115.4	-134.9	-134.9

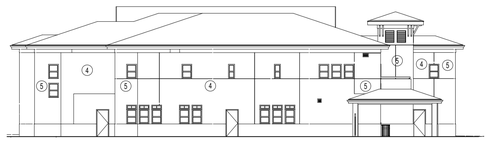
a=6'-0"

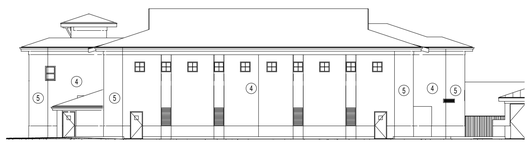
WIND PRESSURES (PSF)
EXTERIOR DOORS, WINDOWS, WALLS

EFFECTIVE AREA (ft)	ZONE 4		ZONE 5	
	PRESSURE	SUCTION	PRESSURE	SUCTION
1 TO 20	72.4	-78.5	72.4	-96.9
21 TO 50	69.1	-75.3	69.1	-90.4
51 TO 100	64.8	-70.9	64.8	-81.8
101 TO 150	61.6	-67.7	61.6	-75.3
151 TO 250	59.7	-65.8	59.7	-71.4
APPARATUS BAY 14x14 DOOR	58.4	-64.5	58.4	-68.9

ZONE 5 IS WITHIN 6FT OF ANY CORNER, ALL OTHER AREAS ARE ZONE 4.







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SOWARDS
AGUILA
ARCHITECTS

Architects, Planners
& Interior Designers

AA26001584
134 N.E. 1st Avenue
Delray Beach, Florida 33444
TEL: 561 276-4951
FAX: 561 243-8184
E-mail: Office@CSA-Architects.com

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AAC (RESUBMITTAL)..... 03.21.2012
P&Z 05.01.2012
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PERMIT.....08.06.2012
CONSTRUCTION .....

SEAL

PROJECT TITLE

CITY OF
POMPANO BEACH
FIRE STATION 103

3721 N.E. 12th AVE.
POMPANO BEACH, FL 33062

REVISIONS

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FILE NUMBER

DRAWING TITLE

STRUCTURAL SPECIFICATIONS & WIND LOAD SCHEDULES

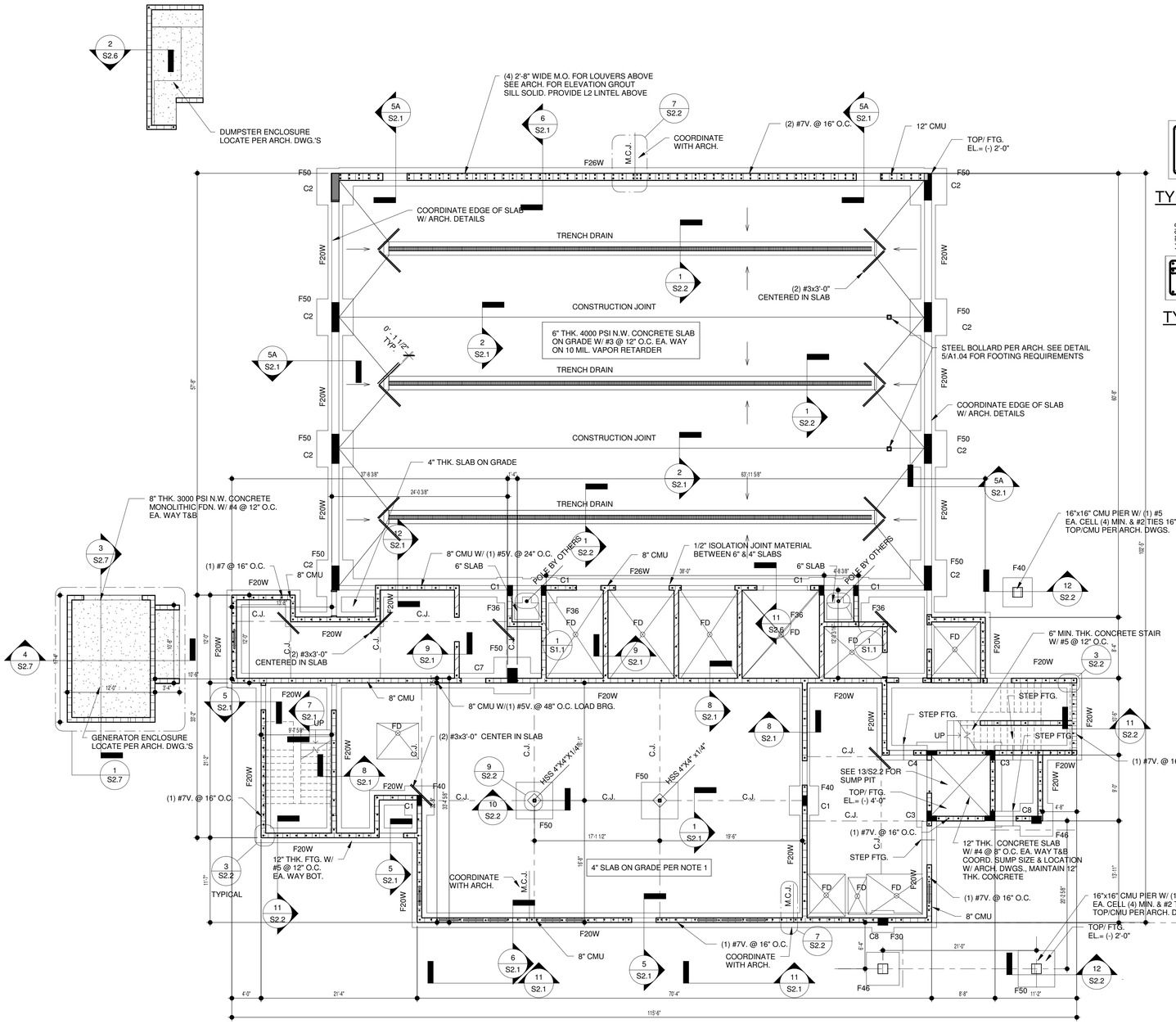
08.06.2012 BID-PERMIT SET

DATE 08.06.2012 | DRAWN BY NM

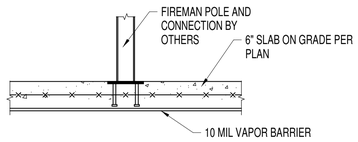
JOB NUMBER 110102

DRAWING NUMBER

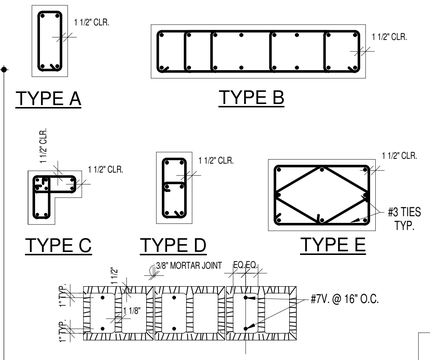
S0.2



FOUNDATION PLAN
1/8" = 1'-0"



1 BASE SLAB SECTION AT FIREMAN POLE (2) PLACES
3/4" = 1'-0"



12" CMU REINFORCING

MASONRY REINF. LAP SCHEDULE

BAR SIZE	LAP LENGTH
#3 BAR	18"
#4 BAR	24"
#5 BAR	30"
#6 BAR	40"
#7 BAR	54"

CONCRETE COLUMN SCHEDULE

MARK	SIZE	REINFORCING VERT.	REINFORCING TIES	REMARKS
C1	8"X16"	(4) #6	#3 @ 8"	TYPE A
C2	12"x48"	(16) #8	#3 @ 12"	TYPE B
C3	8"x16"x16"	(8) #5	#3 @ 8"	TYPE C
C4	8"x16"	(6) #6	#3 @ 8"	TYPE D
C5	8"x12"	(4) #6	#3 @ 8"	TYPE A
C6	8"x20"x20"	(8) #5	#3 @ 8"	TYPE C
C7	16"x24"	(8) #7	#3 @ 12"	TYPE E
C8	12"x16"	(4) #7	#3 @ 12"	TYPE A

FOOTING SCHEDULE

MARK	SIZE	DEPTH	REINF. EA. WAY	REMARKS	DWL/A.B. EMBEDMENT
F20W	2'-0"	1'-0"	(3) #5 CONT. #4@24" TRANSV.	WALL FTG.	9"
F26W	2'-6"	1'-0"	(3) #5 CONT. #5@16" TRANSV.	WALL FTG.	9"
F30	3'-0"x3'-0"	1'-0"	(3) #5		9"
F36	3'-6"x3'-6"	1'-0"	(4) #5		9"
F40	4'-0"x4'-0"	1'-2"	(4) #5		11"
F46	4'-6"x4'-6"	1'-0"	(4) #5		9"
F50	5'-0"x5'-0"	1'-3"	(6) #5		1'-0"

CONCRETE COLUMN VERTICAL REINFORCEMENT BAR LAP SCHEDULE

BAR SIZE	COMPRESSION LAP	CLASS "B" TENSION LAP 3,000 PSI	4,000 PSI	5,000 PSI
# 5	25"	36"	31"	28"
# 6	30"	43"	37"	33"
# 7	35"	63"	54"	49"

BASED ON NORMAL WEIGHT CONCRETE & GRADE 60 REINFORCING BARS

CONCRETE COLUMN VERTICAL REINFORCEMENT BAR LAP SCHEDULE
3/4" = 1'-0"

FOUNDATION PLAN NOTES

1. SLAB-ON-GRADE TO BE 4" THICK, 3000 PSI CONC., REINF. W/ 6x6 - W1.4xW1.4 W.W.F. U.N.O. ON 10 MIL VAPOR RETARDER, LAP AND TAPE OVER COMPACTED SOIL. REFER TO ARCH. FOR EXTENT OF SLAB. SEE ARCH. SPECIFICATION FOR FINISH REQUIREMENTS.
2. SLAB-ON-GRADE IN APPARATUS BAY TO BE 6" (4000 PSI) W/ #3 BARS @ 12" O.C. EACH WAY OVER 10 MIL VAPOR RETARDER LAP & TAPE 6". COMPACT SUBGRADE PER GEOTECH REPORT. SEE ARCH. SPECIFICATIONS FOR FINSH REQUIREMENTS.
3. TOP OF SLAB = 0'-0" U.N.O
   TOP EXTERIOR OF FOOTING = (-) 2'-0" U.N.O.
   TOP/ INTERIOR FTG.= (-) 2'-0" U.N.O.
4. CENTER ALL FOOTINGS BELOW WALL/COLUMN U.N.O.
5. ALL FOOTING REINFORCING TO BE BOTTOM BARS U.N.O.
6. INDICATES 8" OR 12" NOMINAL MASONRY WALLS REINFORCE PER PLAN IN FULLY GROUTED CELLS & AT ALL WALL INTERSECTIONS, SIDES OF OPENINGS AND AT CORNERS W/ MATCHING DOWELS INTO FOUNDATION CONT. UP THROUGH TIE BEAM @ FLOOR ELEVATION & HOOK INTO TIE BEAM @ ROOF ELEV., TYP. SEE 9/S2.1
   INDICATES 8" NON-BEARING MASONRY W/ #5 @ 4'-0" O.C. MAX
7. REFER TO SHEETS S0.1, S0.2 AND S0.3 FOR SPECIFICATIONS.
8. VERIFY ALL DIMENSIONS W/ ARCH. PRIOR TO FABRICATION & CONSTRUCTION. SEE ARCH DRAWINGS FOR MASONRY OPENING SIZE & LOCATION.
9. VERIFY FOOTING ELEVATIONS W/ CIVIL. MAINTAIN 12" SOIL COVER ON TOP OF FOOTING. TYP., REFER TO 3/S2.1
10. CJ = CONTRACTION JOINT SEE 1/ S2.1
11. M.C.J. = VERIFY ALL MASONRY CONTROL JOINT (MCJ) LOCATIONS WITH SPECIFICATIONS AND ARCHITECT. SEE 7/S2.2
12. ALL ARCHITECTURALLY EXPOSED MASONRY SHALL BE CONSTRUCTED WITH SMOOTH, NON-BROKEN, UNCHIPPED BLOCKS WITH FLUSH STRUCK MORTAR JOINTS.
13. FD= FLOOR DRAIN SEE PLUMBING DRAWINGS
14. SEE S2.1 & S2.2 FOR TYPICAL FOUNDATION DETAILS

McCarthy and Associates, Inc.
CONSULTING ENGINEERS
www.mccarthyassoc.com
601 N. CONGRESS AVE. SUITE 106A
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SEAL

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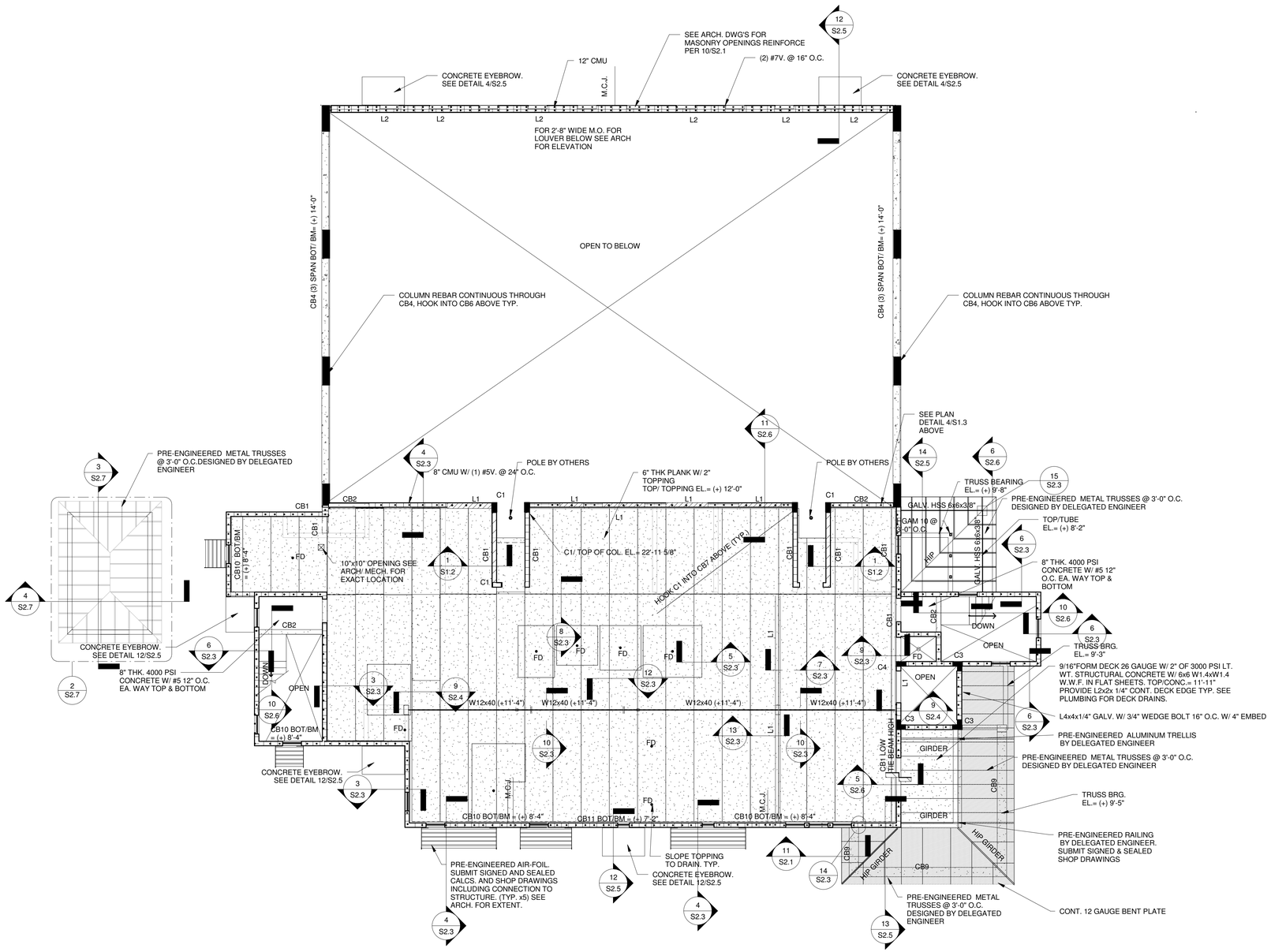
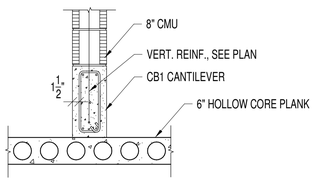
FILE NUMBER

DRAWING TITLE
FOUNDATION PLAN
SCALE= 1/8" = 1'-0"

DATE 08.06.2012 | DRAWN BY NM
JOB NUMBER 110102
DRAWING NUMBER
S1.1

08.06.2012 BID-PERMIT SET





1 WALL/PLANK SECTION

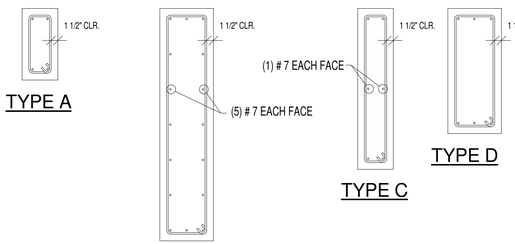
3/4" = 1'-0"

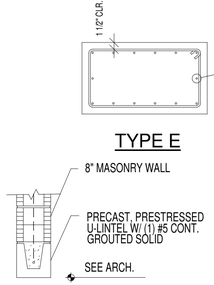
FLOOR FRAMING PLAN NOTES

1. FLOOR FRAMING TO BE 6" HOLLOW CORE PLANK W/ 2" BONDED COMPOSITE TOPPING MIN. @ INTERIOR AREA.
   FIN. FLOOR EL. = (+) 12'-0" U.N.O.
   PLANK BEARING EL. = (+) 11'-4"
2. FLOOR TOPPING TO BE 2" (3000 PSI) NORMAL WEIGHT CONCRETE TOPPING W/ 6x6-W1.4xW1.4 WWF FURNISHED IN SHEETS, EXCEPT WHERE NOTED OTHERWISE.
3. PRECAST PLANK JOINTS INDICATE THE DIRECTION OF SPAN. JOINT SPACING IS SHOWN FOR GRAPHIC PURPOSES ONLY. PRECAST MANUFACTURER TO DETERMINE THE EXACT SPACING OF THE PRECAST PLANKS.
4. VERIFY ALL BEARING ELEVATIONS WITH ARCHITECTURAL DRAWINGS PRIOR TO FABRICATION.
5. REFER TO S0.1 , S0.2 & S0.3 FOR SPECIFICATIONS.
6. COORDINATE MASONRY CONTROL JOINTS W/ ARCH'L DRAWINGS. SEE 7/S2.2 FOR ADDITIONAL INFORMATION.
7. SLOPE EXTERIOR SLABS TO DRAIN AWAY FROM BUILDING.
8. REFER TO S1.1 FOR CONCRETE COLUMN SCHEDULE.
9. PRE-CAST SLAB END BEARING= 3 1/2" MIN. U.N.O.
   PRE-CAST SIDE BEARING= 2" MIN. U.N.O.
10. GROUT ALL CAMBER GAPS AFTER POURING TOPPING.

CONCRETE BEAM SCHEDULE

MARK	WIDTH x DEPTH	REINFORCING TOP	REINFORCING BOT.	STIRRUPS	REMARKS
CB1	8"x16"	(2) # 5	(2) # 5	# 3 @ 6"	TYPE A
CB2	8"x16"	(2) # 6	(2) # 6	# 3 @ 6"	TYPE A
CB3	8"x16"	(2) # 7	(2) # 7	# 3 @ 6"	TYPE A
CB4	12"x52"	(3) # 7	(3) # 7	# 3 @ 12"	TYPE B
CB5	8"x36"	(2) # 7	(2) # 7	# 3 @ 12"	TYPE C
CB6	12"x28"	(3) # 7	(3) # 7	# 3 @ 12"	TYPE D
CB7	28"x16"	(6) # 6	(6) # 6	# 3 @ 8"	TYPE E
CB8	18"x16"	(4) # 6	(4) # 6	# 3 @ 8"	TYPE F
CB9	16"x16"	(4) # 7	(4) # 7	# 3 @ 6"	TYPE F
CB10	8"x34"	(2) # 7	(2) # 7	# 3 @ 12"	TYPE C
CB11	8"x48"	(2) # 7	(2) # 7	# 3 @ 12"	TYPE C
CB12	8"x26"	(2) # 7	(2) # 7	# 3 @ 12"	TYPE C





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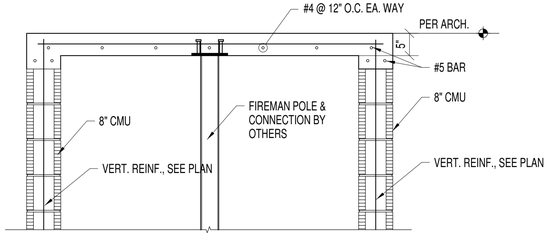
FILE NUMBER

DRAWING TITLE
SECOND FLOOR PLAN & LOWER ROOF FRAMING PLAN
SCALE= 1/8" = 1'-0"

DATE 08.06.2012 | DRAWN BY NM
JOB NUMBER 110102
DRAWING NUMBER
S1.2

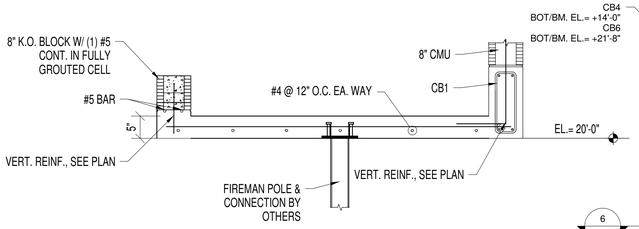
08.06.2012 BID-PERMIT SET

8/7/2012 12:34:08 PM



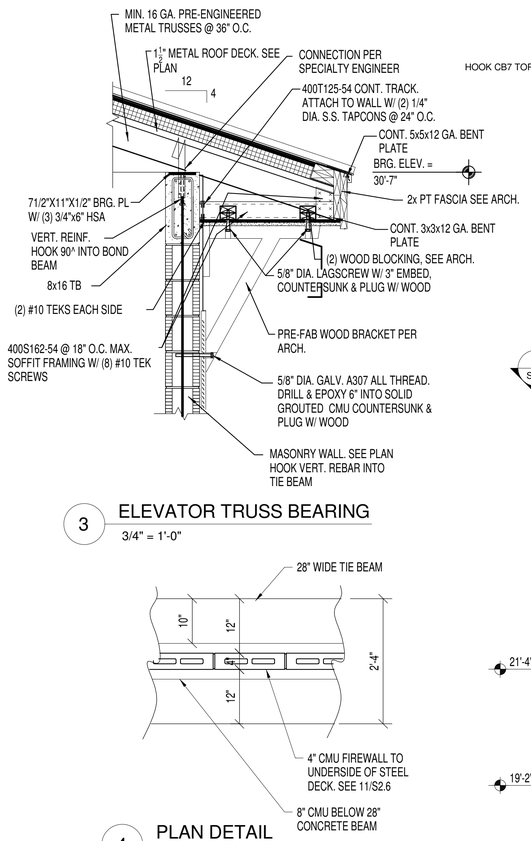
1 SLAB SECTION AT FIREMAN POLE (2) PLACES

3/4" = 1'-0"



2 SLAB SECTION AT FIREMAN POLE (2) PLACES #2

3/4" = 1'-0"

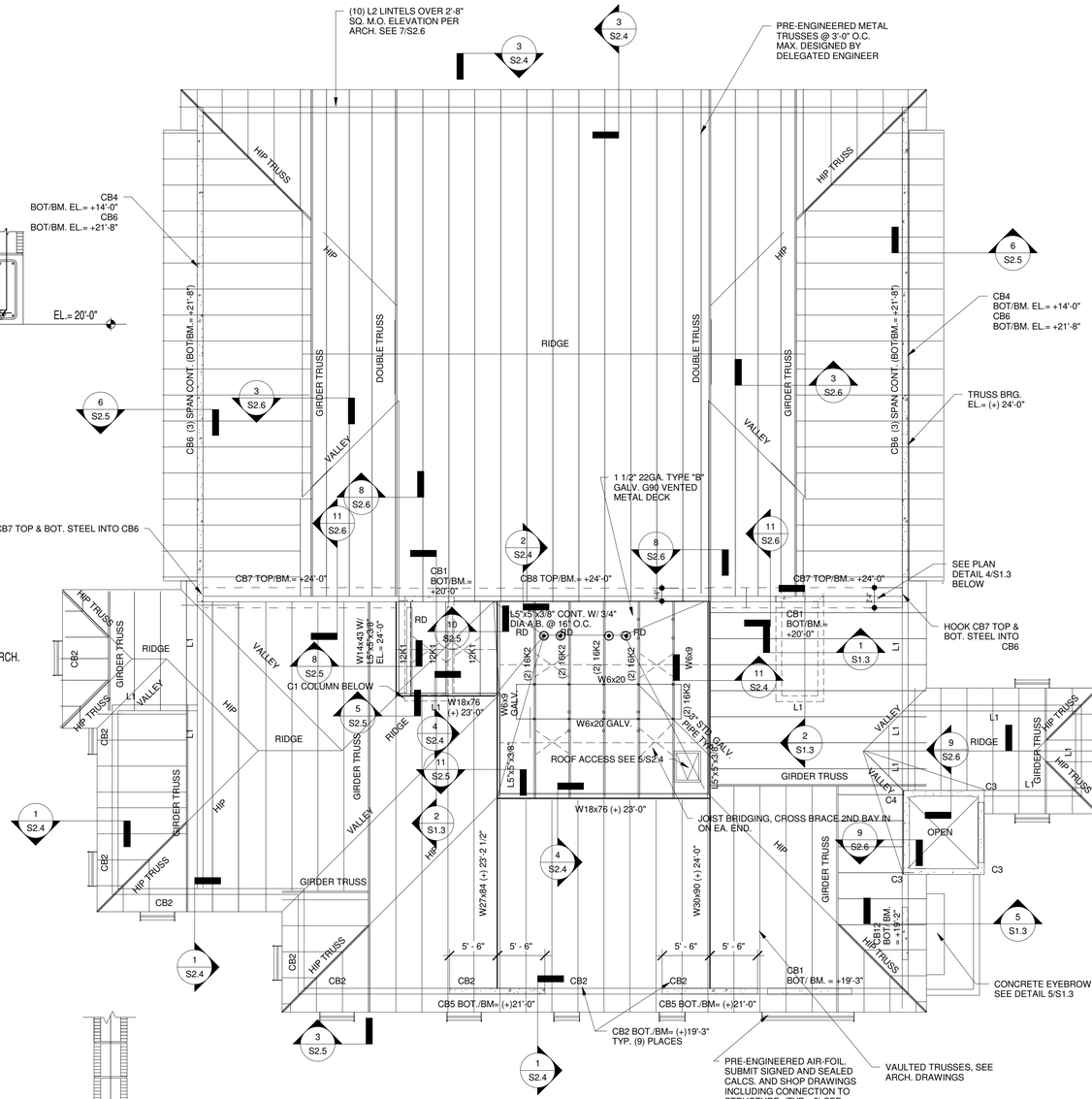


3 ELEVATOR TRUSS BEARING

3/4" = 1'-0"

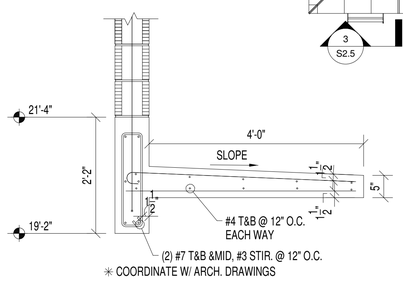
4 PLAN DETAIL

3/4" = 1'-0"



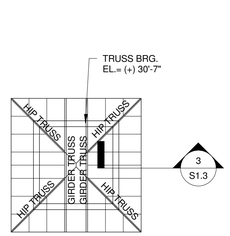
ROOF FRAMING PLAN

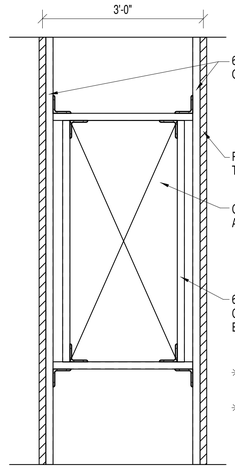
1/8" = 1'-0"



5 CONCRETE EYEBROW EAST

3/4" = 1'-0"





6 MECH. LOUVER OPENING FRAMING

3/4" = 1'-0"

ROOF FRAMING PLAN NOTES

1. ROOF FRAMING TO BE PRE-ENGINEERED METAL TRUSSES EQUALLY SPACED BETWEEN SUPPORTS @ 36" O.C. U.N.O. PROVIDE BRIDGING PER MFR. TRUSS SPECIALTY ENGINEER.
2. WIND UPLIFT = SEE UPLIFT TABLE ON SPECIFICATION SHEET S0.3.
3. METAL ROOF DECK TO BE 1 1/2" DP. (18) GA., GALVANIZED TYPE "B" INSTALLED (3) SPAN MIN. FASTEN DECK W/ 36/7 WELD PATTERN AT EACH SUPPORT AND (7) #10 TEK STITCH SCREWS SIDELAP FASTENERS FOR 36" WIDE DECKING.
4. ROOF SLOPE= 4/12 U.N.O.
5. ( )= TOP OF STEEL ELEVATION.
6. COORDINATE ALL DIMENSIONS/ELEVATIONS W/ ARCHITECT BEFORE STARTING CONSTRUCTION.
7. G.C./ ERECTOR IS RESPONSIBLE FOR ALL TEMPORARY BRACING.
8. REFER TO S2.4 & S2.5 FOR TYPICAL ROOF FRAMING DETAILS.
9. CB ## = 4000 PSI, P.I.P. CONC. BEAM. SEE SCHED'L ON S1.2 BOTTOM OF BEAM EQUALS TOP OF OPENING U.N.O.
10. REFER TO SHEET S0.1, S0.2 & S0.3 FOR SPECIFICATIONS
11. TRUSS BEARING ELEVATION= (+) 24'-0"

HIGH FLAT ROOF FRAMING PLAN NOTES

1. METAL ROOF DECK TO BE 1 1/2" 22GA. TYPE "B" GALV. G90 VENTED DECK FASTEN DECK W/ 36/7 PUDDLE WELD AT EACH SUPPORT AND (7) #10 TEK SCREW SIDELAP FASTENERS FOR 36" WIDE DECKING.
2. WIND UPLIFT = SEE UPLIFT DIAGRAM ON DWG. SO.3
3. JOISTS ARE EQUALLY SPACED BETWEEN SUPPORTS U.N.O. AT NOT MORE THAN 4'-0" O.C. MAX.
4. (+) = TOP/ STEEL BEAM ELEV.= JOIST BEARING
5. REFER TO SHEETS S2.5 FOR TYPICAL FRAMING DETAILS.
6. REFER TO SHEET S0.1 THRU S0.3 FOR SPECIFICATIONS
7. REFER TO SHEET S1.2 FOR CONCRETE BEAM SCHEDULE
8. REFER TO ARCH. FOR EXTENT OF ALL PARAPETS (LOCATION, ELEVATIONS, STEPS, ETC.)
9. INDICATES DECK SPAN DIRECTION.

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P&Z 05.01.2012
BIDS..... 08.06.2012
PERMIT..... 08.06.2012
CONSTRUCTION .....

SEAL

PROJECT TITLE

CITY OF POMPANO BEACH FIRE STATION 103

3721 N.E. 12th AVE.
POMPANO BEACH, FL 33062

REVISIONS

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FILE NUMBER

DRAWING TITLE

ROOF FRAMING PLAN

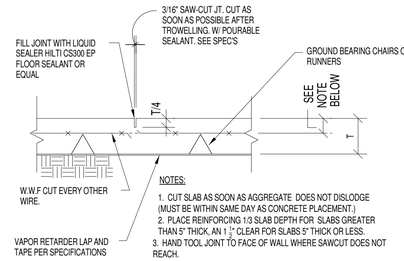
DATE 08.06.2012 | DRAWN BY NM

JOB NUMBER 110102

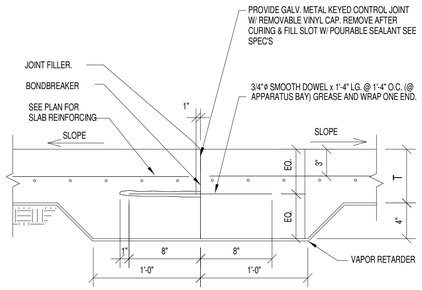
DRAWING NUMBER

S1.3

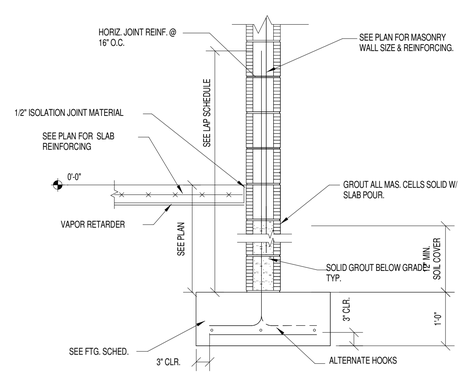
08.06.2012 BID-PERMIT SET



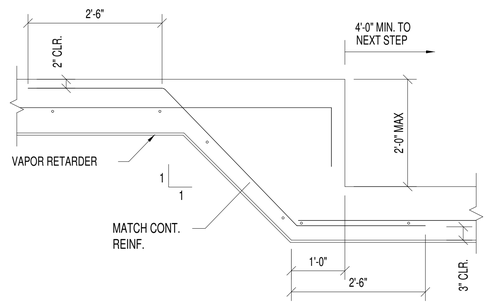
1 CONTRACTION JOINT
3/4" = 1'-0"



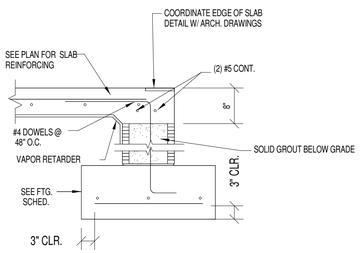
2 CONSTRUCTION JOINT IN APPARATUS BAY
3/4" = 1'-0"



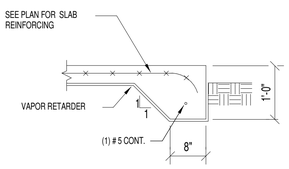
3 WALL FOOTING
3/4" = 1'-0"



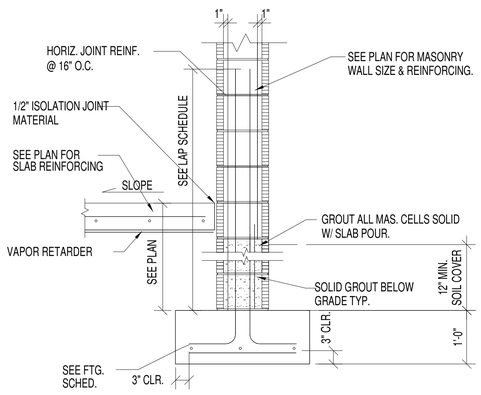
4 STEPPED FOOTING
3/4" = 1'-0"



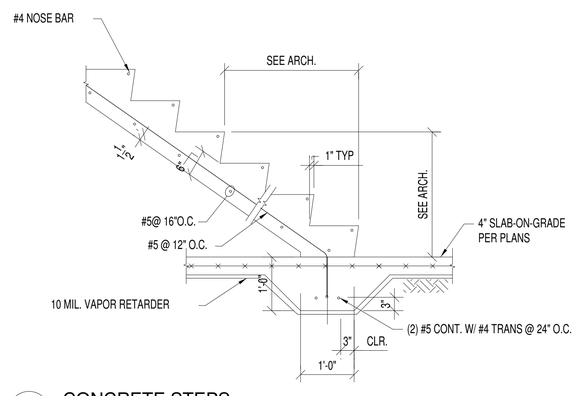
5A SLAB EDGE at 12" CMU
3/4" = 1'-0"



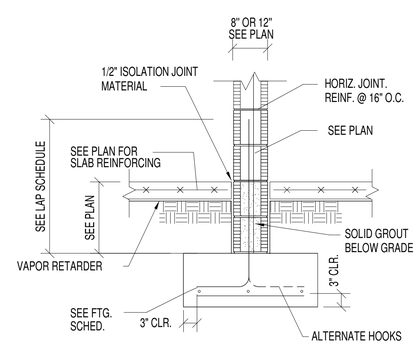
5B THICKENED SLAB EDGE
3/4" = 1'-0"



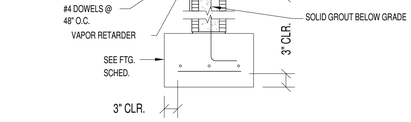
6 WALL FOOTING AT 12" CMU
3/4" = 1'-0"



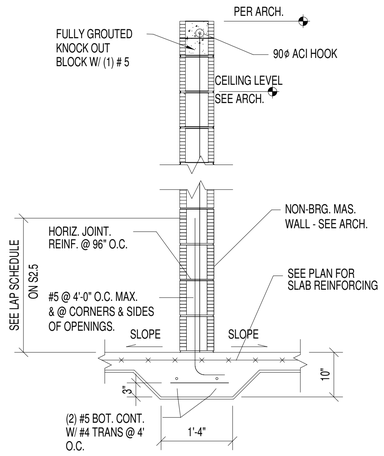
7 CONCRETE STEPS
3/4" = 1'-0"



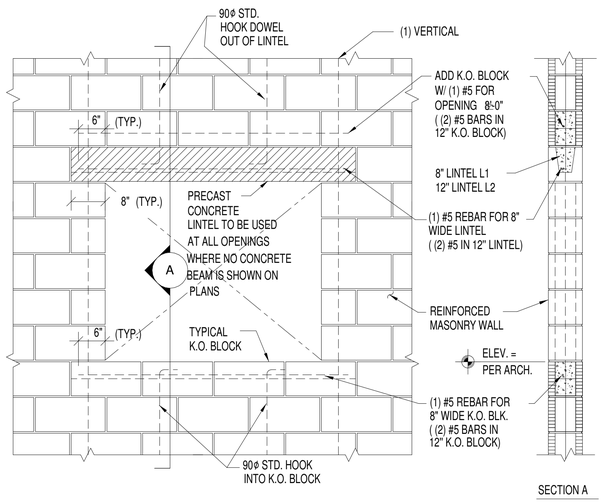
8 INTERIOR WALL FOOTING
3/4" = 1'-0"



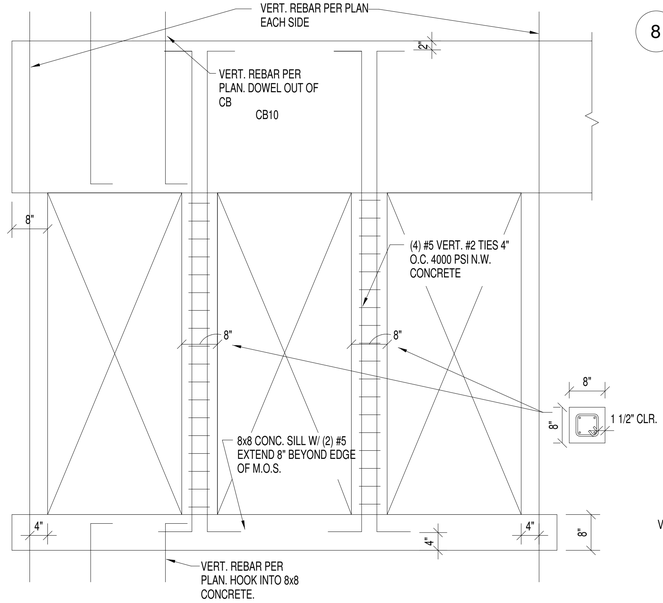
5 SLAB EDGE
3/4" = 1'-0"



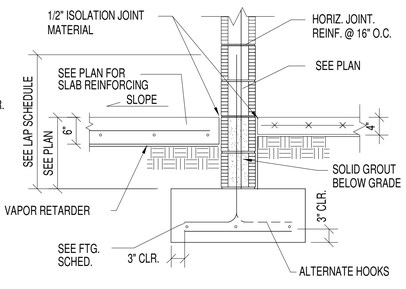
9 INTERIOR NON-BRG. MASONRY
3/4" = 1'-0"



10 TYPICAL CONCRETE LINTEL REINFORCING DETAIL
3/4" = 1'-0"



11 3 BAY WINDOW 2 BAY SIMILAR
3/4" = 1'-0"



12 INTERIOR WALL FOOTING SLAB TRANS
3/4" = 1'-0"

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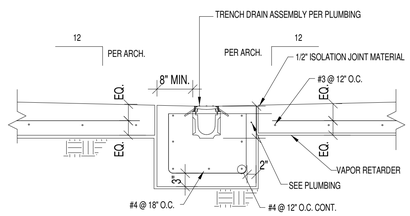
FILE NUMBER

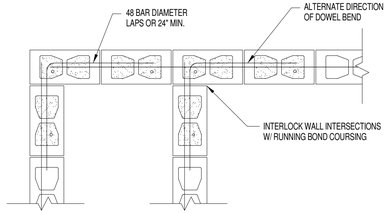
DRAWING TITLE
FOUNDATION DETAILS

DATE 08.06.2012 | DRAWN BY NM
JOB NUMBER 110102
DRAWING NUMBER
S2.1

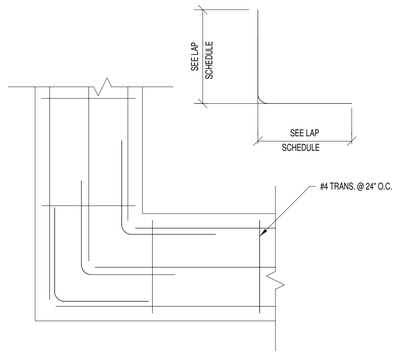
08.06.2012 BID-PERMIT SET



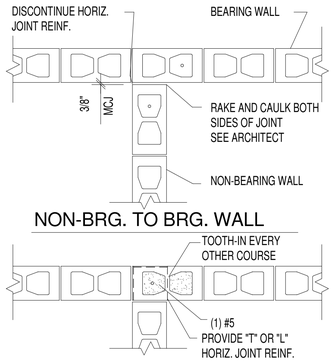
1 TRENCH DRAIN
3/4" = 1'-0"



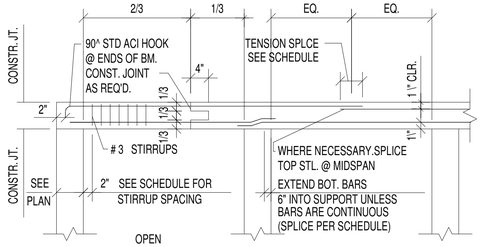
2 8" MASONRY BOND BEAM CONNECTION
3/4" = 1'-0"



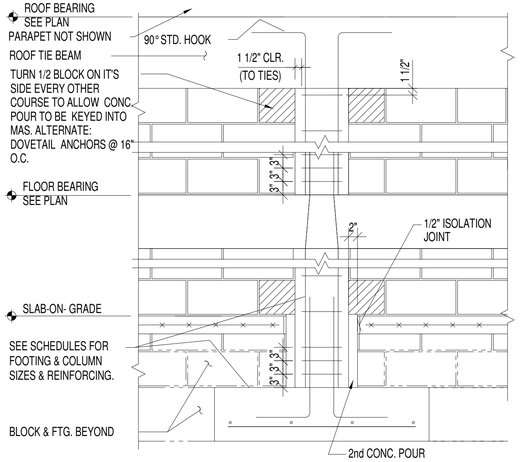
3 CORNER BAR DETAIL AT FOUNDATION
3/4" = 1'-0"



4 TYP. WALL INTERSECTIONS
3/4" = 1'-0"

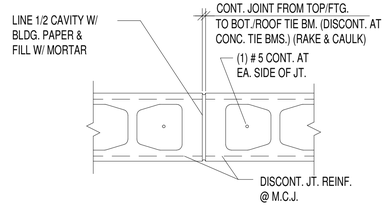


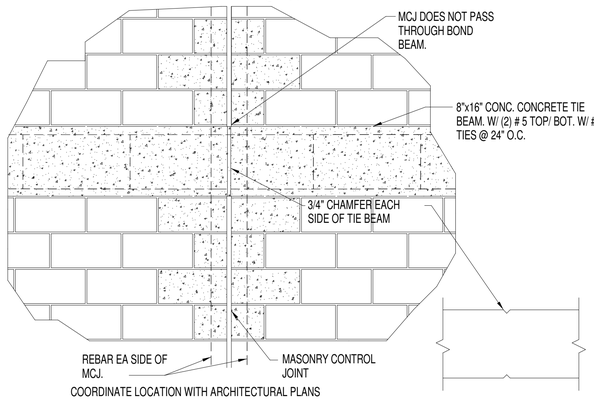
5 BEAM REINFORCING
3/4" = 1'-0"



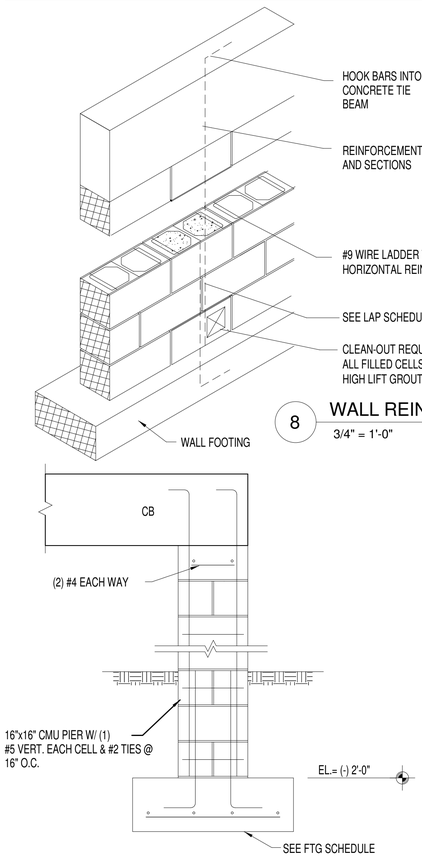
6 TYP. MULTI STORY CONCRETE COLUMN
3/4" = 1'-0"

NOTES
1. EXTEND JOINT VERTICALLY FROM TOP OF FOOTING TO ROOF.
2. DISCONTINUE AT INTERMEDIATE TIE BEAMS AND ROOF TIE BEAMS.
3. SCORE ALL TIE BEAMS ON BOTH SIDES OF WALL TO MATCH JOINT.
4. SPACE JOINTS PER PLAN.
5. COORDINATE LOCATIONS OF JOINTS WITH ARCH. JOINT LOCATIONS.





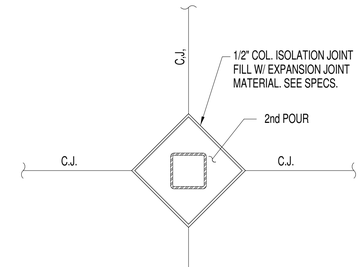
7 MASONRY CONTROL JOINT ELEVATION
3/4" = 1'-0"



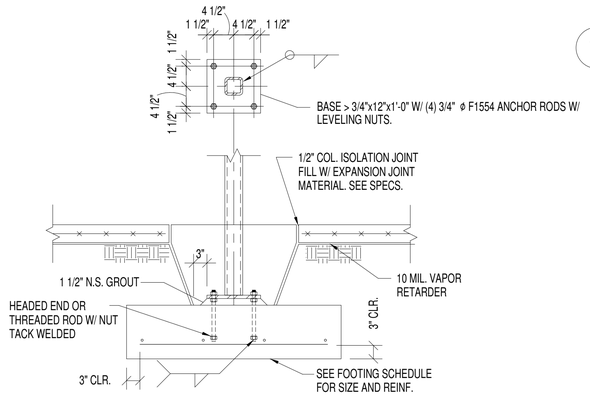
8 WALL REINFORCING
3/4" = 1'-0"

LOW LIFT GROUTING PROCEDURE:
1. CONSTRUCT WALL TO HEIGHT OF 5'-0" ALLOW MORTAR TO SET SUFFICIENTLY TO WITHSTAND GROUT PRESSURE.
2. INSPECT UNITS FOR ALIGNMENT, CLEAN OUT CELLS TO BE FILLED.
3. FILL CELLS TO 1 1/2" BELOW TOP COURSE.
4. DELAY 3 TO 5 MINUTES PRIOR TO CONSOLIDATING TO ALLOW WATER TO BE ABSORBED BY MASONRY.

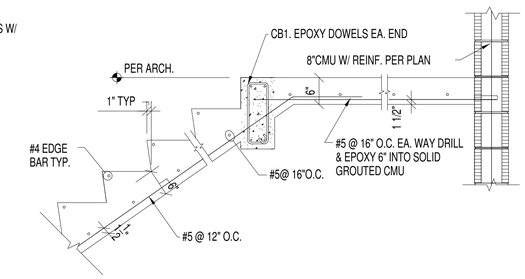
HIGH LIFT GROUTING PROCEDURE:
1. CONSTRUCT WALL TO FULL HEIGHT (24 FEET MAX.) ALLOW MASONRY TO CURE AT LEAST 3 DAYS.
2. CLEAN CELLS, WHICH ARE TO BE GROUTED THROUGH CLEAN-OUT PORTS.
3. PLACE GROUT IN 4 FOOT LIFTS AND CONSOLIDATE AFTER EXCESS MOISTURE HAS BEEN ABSORBED BY MASONRY.
4. PLACE THE NEXT LIFT AS SOON AS POSSIBLE BUT NO LONGER THAN ON HOUR LATER.



9 COL. ISOLATION JOINT
3/4" = 1'-0"

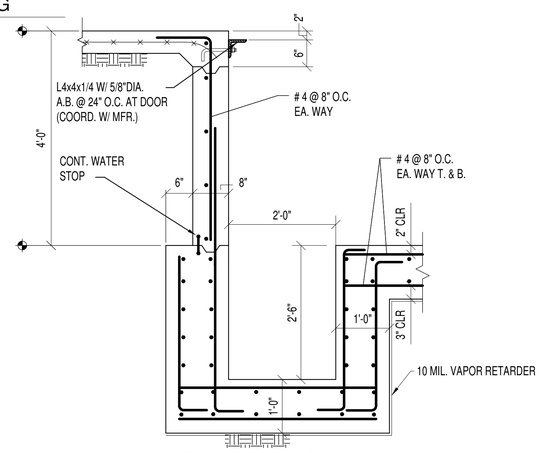


10 COLUMN FOOTING
3/4" = 1'-0"



11 STAIR LANDING
3/4" = 1'-0"

12 CMU PIER ELEVATION
3/4" = 1'-0"



13 ELEVATOR SUMP PIT
3/4" = 1'-0"

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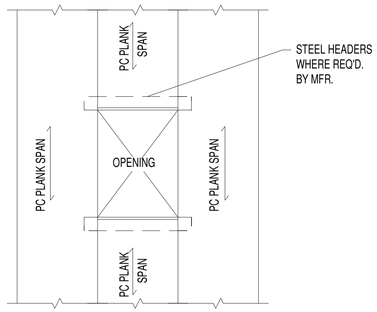
FILE NUMBER

DRAWING TITLE
MASONRY DETAILS

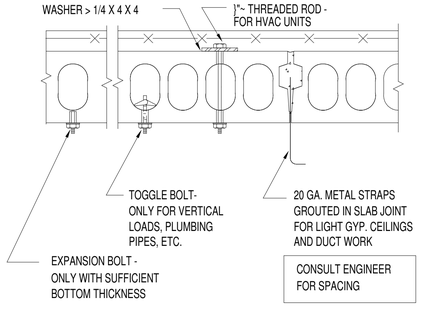
DATE 08.06.2012 | DRAWN BY NM
JOB NUMBER 110102
DRAWING NUMBER
S2.2

08.06.2012 BID-PERMIT SET

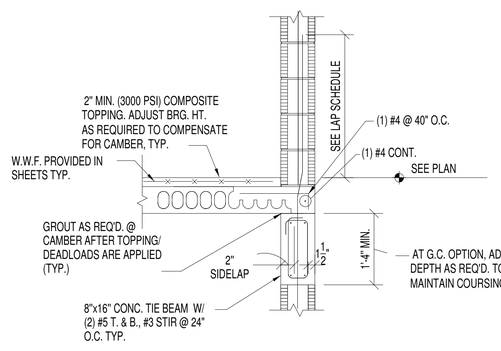
8/1/2012 1:54:20 PM



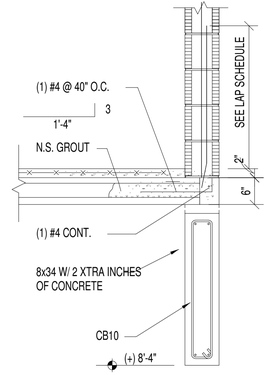
1 TYP. PRECAST OPENING
3/4" = 1'-0"



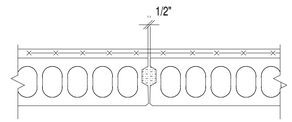
2 HANGER DETAILS
3/4" = 1'-0"



3 PRECAST SIDELAP
3/4" = 1'-0"



4 PRECAST BEARING
3/4" = 1'-0"



NOTE:
ALL JOINTS BETWEEN SLABS SHALL BE FULLY PACKED WITH STIFF GROUT CONSISTING OF (1) PART PORTLAND CEMENT TO (3) PARTS CLEAN SAND.

5 GROUTED KEYWAY TYP.
3/4" = 1'-0"

McCarthy and Associates, Inc.
CONSULTING ENGINEERS
www.mccarthyassoc.com
601 N. CONGRESS AVE. SUITE 106A
Delray Beach, FL. 33445
(561)265-6864
Florida Coa 4187
Robert J. Selinsky, P.E.
Florida P.E. 49952
McCarthy Project No. 11261

CURRIE
SOWARDS
AGUILA
ARCHITECTS

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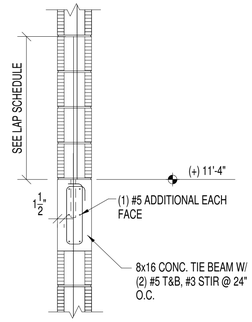
AA26001584
134 N.E. 1st Avenue
Delray Beach, Florida 33444
TEL: 561 276-4951
FAX: 561 243-8184
E-mail: Office@CSA-Architects.com

ISSUED FOR :
DRC 11.29.2011
AAC 01.18.2012
AAC (RESUBMITTAL)..... 03.21.2012
P&Z 05.01.2012
BIDS.....08.06.2012
PERMIT.....08.06.2012
CONSTRUCTION .....

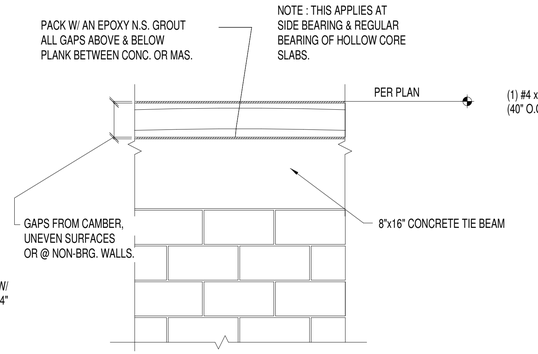
SEAL

PROJECT TITLE

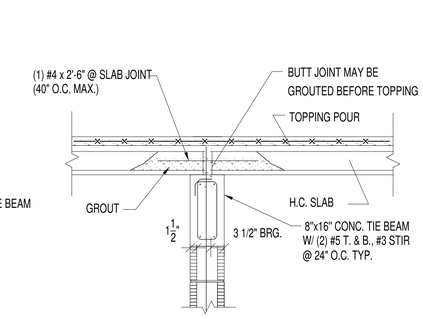
CITY OF
POMPANO BEACH
FIRE STATION 103



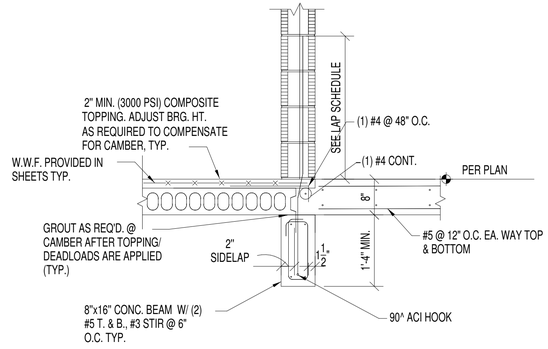
6 TIE BEAM SECTION
3/4" = 1'-0"



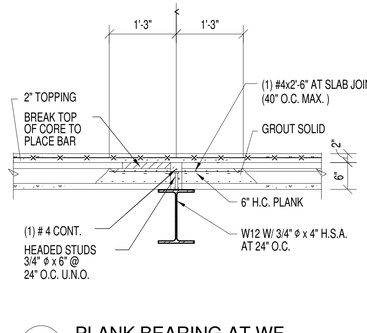
7 PRECAST PLANK BEARING
3/4" = 1'-0"



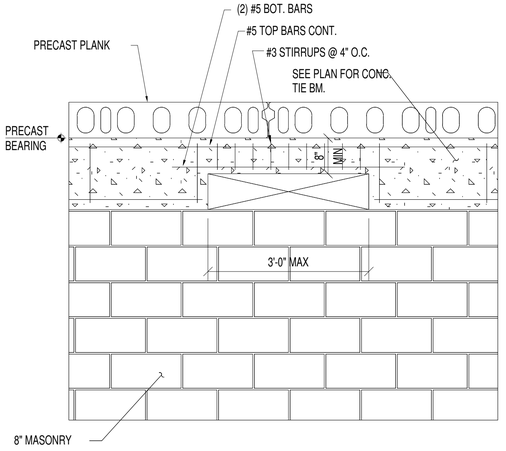
8 INTERIOR PRECAST BEARING
3/4" = 1'-0"



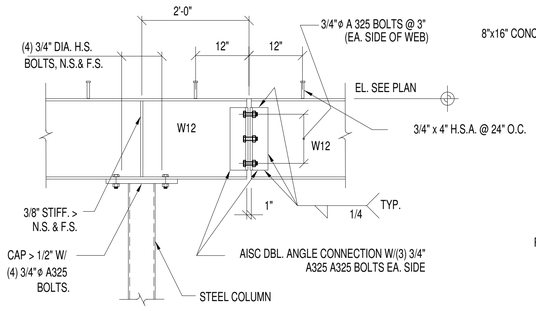
9 INTERIOR PRECAST SIDELAP
3/4" = 1'-0"



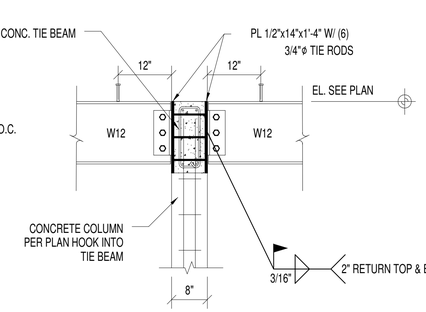
10 PLANK BEARING AT WF
3/4" = 1'-0"



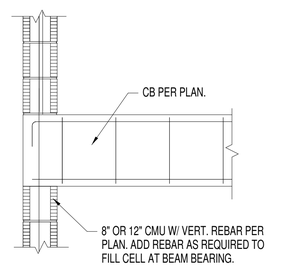
11 MECH. DUCT OPENING
3/4" = 1'-0"



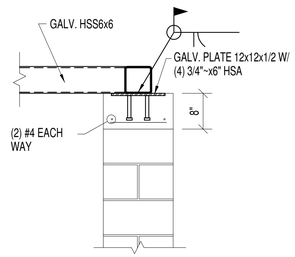
12 BEAM TO COLUMN SPLICE CONNECTION
3/4" = 1'-0"



13 BEAM TO CONCRETE COLUMN CONNECTION
3/4" = 1'-0"



14 TYPICAL CB BEARING
3/4" = 1'-0"



15 CONNECTION
3/4" = 1'-0"

3721 N.E. 12th AVE.
POMPANO BEACH, FL 33062

REVISIONS

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FILE NUMBER

DRAWING TITLE

FRAMING
DETAILS

08.06.2012 BID-PERMIT SET

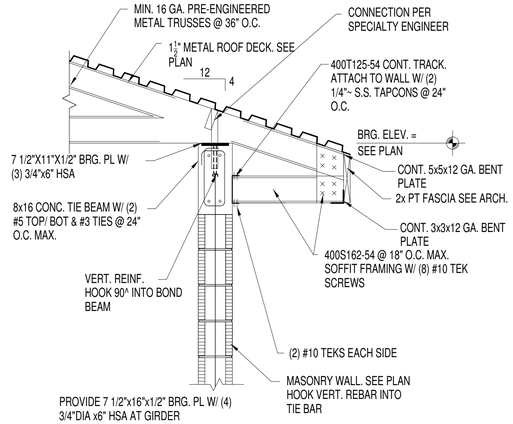
DATE 08.06.2012 | DRAWN BY NM

JOB NUMBER 110102

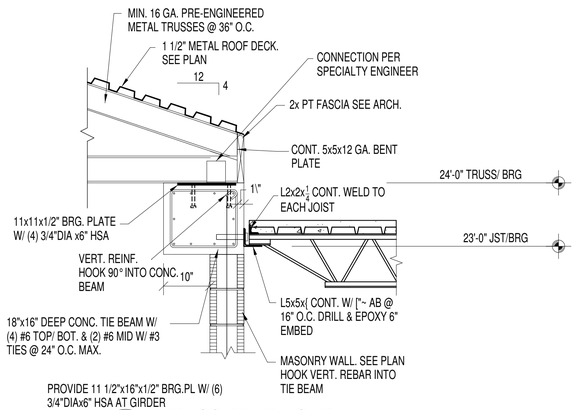
DRAWING NUMBER

S2.3

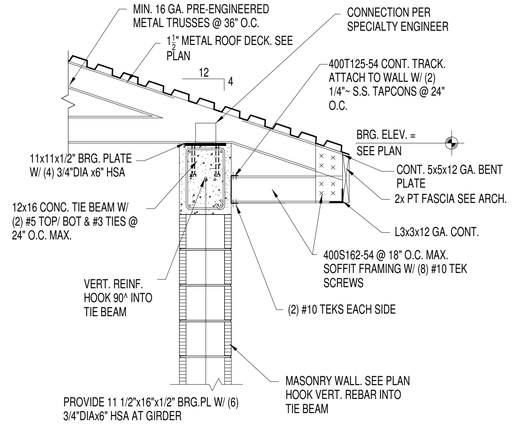
8/1/2012 1:54:21 PM



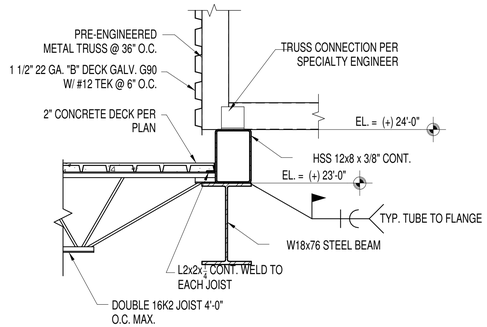
1 TRUSS BEARING #1
3/4" = 1'-0"



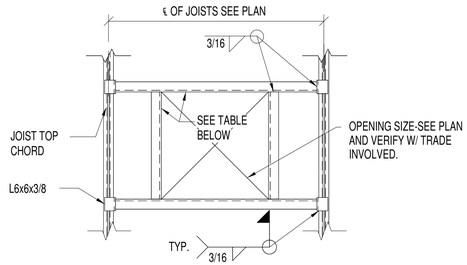
2 TRUSS BEARING #2
3/4" = 1'-0"



3 TRUSS BEARING #3
3/4" = 1'-0"



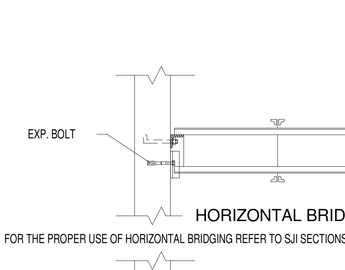
4 TRUSS BEARING #4
3/4" = 1'-0"



1. LOCATE ANGLES BELOW ALL EQUIP. CURBS AND AROUND ALL ROOF OPENINGS.
2. CONTRACTOR SHALL VERIFY LOCATION & SIZE OF OPENINGS PRIOR TO STEEL FABRICATION.
3. FRAME IS REQ'D FOR OPENING 1'-0" & GREATER.

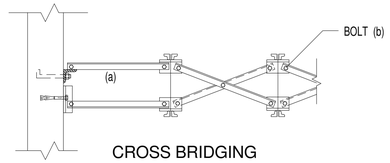
UNIT WEIGHT	ANGLE SIZE
0 - 675 lbs.	L4x3x1/4 L.L.V.
676 - 1500 lbs.	L4x3\x5/16 L.L.V.
1501 - 3000 lbs.	L6x4x3/8 L.L.V.
3001 - 6000 lbs	L6x6x3/8

5 RTU FRAMING DETAIL
3/4" = 1'-0"



HORIZONTAL BRIDGING

FOR THE PROPER USE OF HORIZONTAL BRIDGING REFER TO SJI SECTIONS 104.5 AND 105.

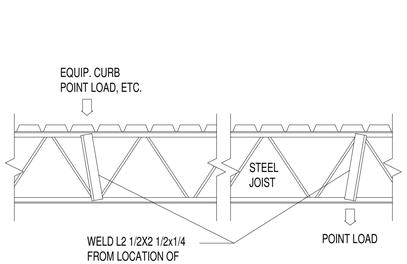


CROSS BRIDGING

(a) HORIZONTAL BRIDGING UNITS SHALL BE USED IN THE SPACE ADJACENT TO THE WALL TO ALLOW FOR PROPER DEFLECTION OF THE JOIST NEAREST THE WALL.

(b) FOR REQUIRED BOLT SIZE REFER TO BRIDGING SPACING TABLE OF SJI.

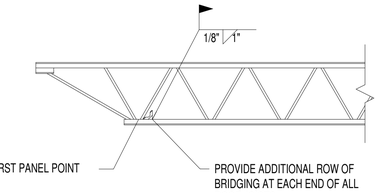
6 BRIDGING ANCHOR DETAILS
3/4" = 1'-0"



NOTE : JST. STIFF.S NOT REQUIRED FOR POINT LOADS :
≦ 250 LBS @ TOP CHORD
≦ 100 LBS @ BOT. CHORD

(WHERE POINT LOADS OCCUR BETWEEN JOIST PANEL POINTS)

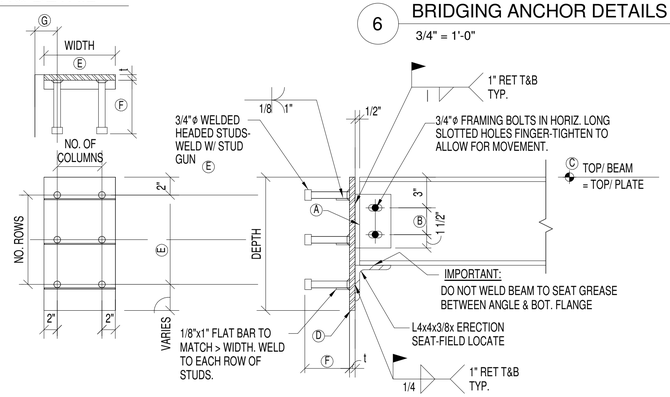
7 JOIST STIFFENER DETAIL
3/4" = 1'-0"



8 TYP. UPLIFT BRIDGING
3/4" = 1'-0"

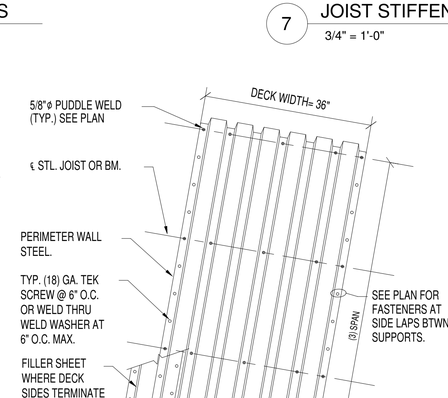
NOTES:

1. EMBED IN CONCRETE.
2. SEE PLATE SIZE FOR MIN. DEPTH CONC. INCREASE TIE BEAM DEPTH AS REQUIRED TO ACCOMODATE FULL PLATE DEPTH. PROVIDE 24" MIN. LENGTH.
3. MIN. CONC. TIE BEAM REINF. WHEN NOT EMBEDDED IN CONC. COL.: 8" WIDE BEAM- (2) #5 @ 8" O.C. 12" WIDE BEAM- (2) #6 @ 8" O.C. & (3) #3 TIES

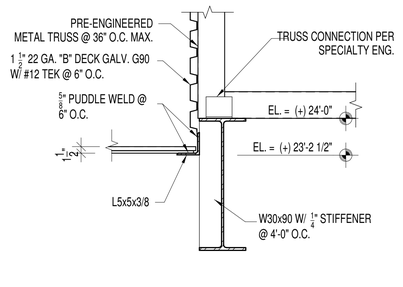


EMBED PLATE SCHEDULE										
BEAM SERIES	MAX REACT. (KIPS)	FRAMING ANGLES	NO. BOLTS	BOLT TYPE	PLATE SIZE (t x W x D)	NO. OF SHEAR STUDS			STUD LENGTH	MIN. EDGE DISTANCE
						COLUMNS	ROWS	TOTAL		
		Ⓐ	Ⓑ	Ⓒ	Ⓓ		Ⓔ		Ⓕ	Ⓖ
W12	52K	(2) L4x4x3/8x9"	3@ 3" O.C.	A325-N	1/2x14x1'-6"	3@ 5" O.C.	3@ 5" O.C.	9	5"	2 1/2"
W14	55K	(2) L4x4x{x9"	3@ 3" O.C.	A325-N	\x14x1'-7"	3@ 5" O.C.	3@ 5" O.C.	9	5"	2\"
W27	130K	(2) L4x4x{x21"	7@ 3" O.C.	A325-N	[x14x2'-9"	3@ 5" O.C.	6@ 5" O.C.	18	5"	2\"
W30	148K	(2) L4x4x3/8x24"	8@ 3" O.C.	A325-N	3/4x14x2'-11"	3@ 5" O.C.	7@ 5" O.C.	21	5"	2 1/2"

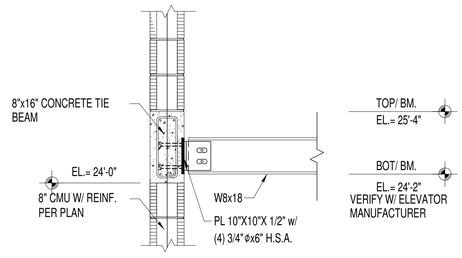
9 BOLT CLUSTER DETAIL
3/4" = 1'-0"



10 METAL ROOF DECK ATTACHMENT
3/4" = 1'-0"



11 TRUSS BEARING
3/4" = 1'-0"



MAXIMUM LOAD= 5,000 LBS

12 ELEVATOR ROOF & HOIST BEAM
3/4" = 1'-0"

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CONSTRUCTION .....

SEAL

PROJECT TITLE

CITY OF POMPANO BEACH FIRE STATION 103

3721 N.E. 12th AVE.
POMPANO BEACH, FL 33062

REVISIONS

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FILE NUMBER

DRAWING TITLE

ROOF FRAMING DETAILS

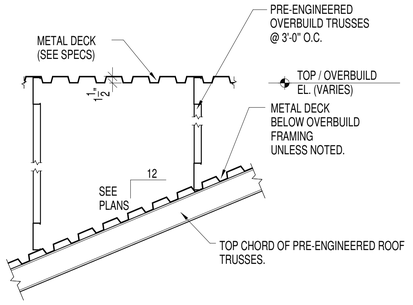
08.06.2012 BID-PERMIT SET

DATE 08.06.2012 | DRAWN BY NM

JOB NUMBER 110102

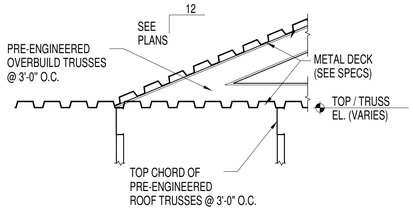
DRAWING NUMBER

S2.4



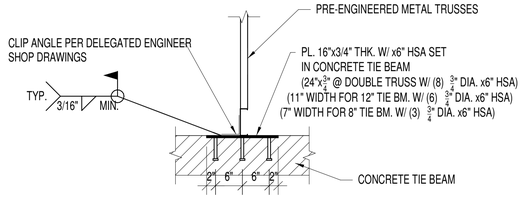
1 OVER BUILT TRUSS CONNECTION

3/4" = 1'-0"



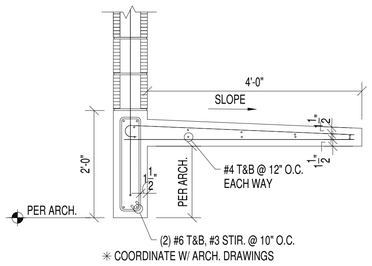
2 OVER BUILT AT VALLEY CONNECTION

3/4" = 1'-0"



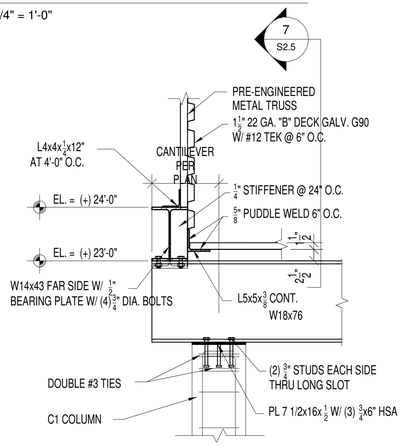
3 TYPICAL METAL TRUSS CONNECTION

3/4" = 1'-0"



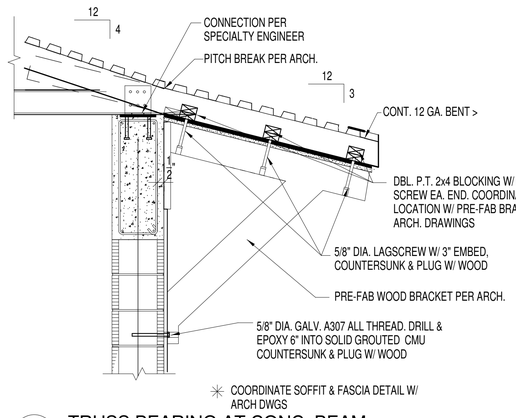
4 CONCRETE EYEBROW NORTH

3/4" = 1'-0"



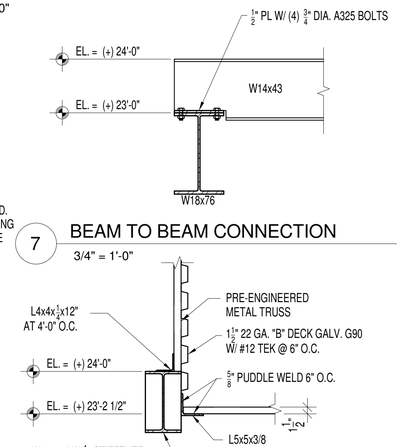
5 BEAM TO COLUMN CONNECTION

3/4" = 1'-0"



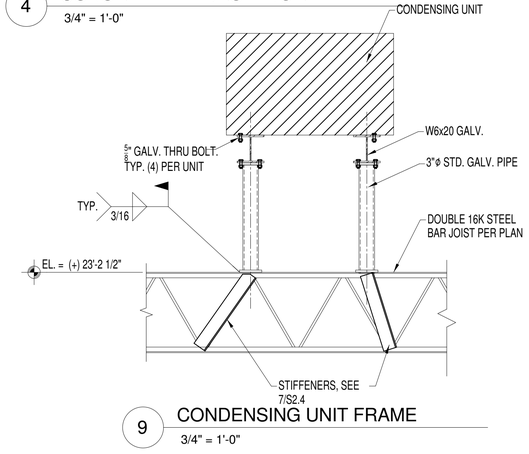
6 TRUSS BEARING AT CONC. BEAM

3/4" = 1'-0"



7 BEAM TO BEAM CONNECTION

3/4" = 1'-0"

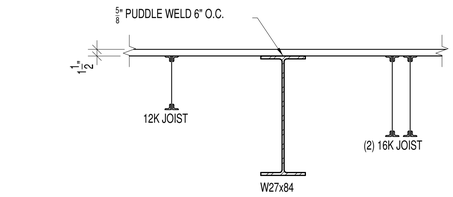


9 CONDENSING UNIT FRAME

3/4" = 1'-0"

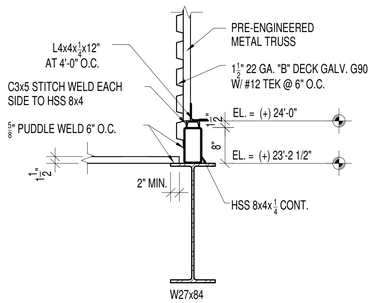
8 BEAM SECTION

3/4" = 1'-0"



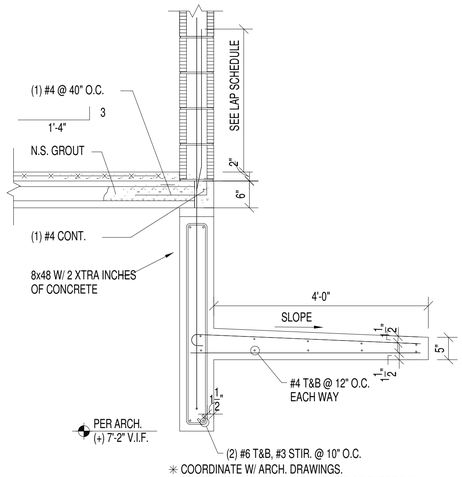
10 DECK BEARING

3/4" = 1'-0"



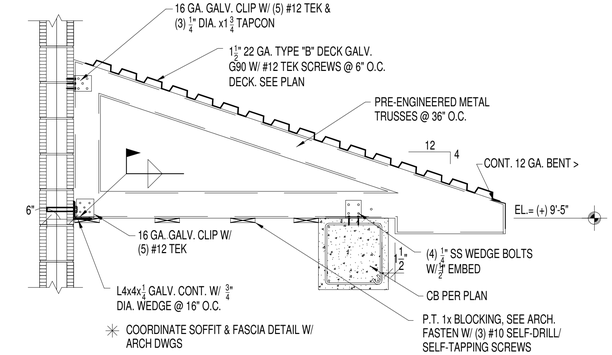
11 WF TO HSS BEAM SECTION

3/4" = 1'-0"



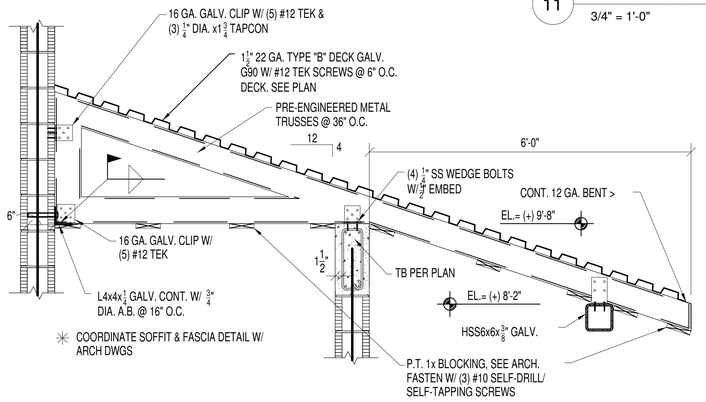
12 CONCRETE EYEBROW

3/4" = 1'-0"



13 WALKWAY TRUSS BEARING

3/4" = 1'-0"



14 WALKWAY TRUSS BEARING #2

3/4" = 1'-0"

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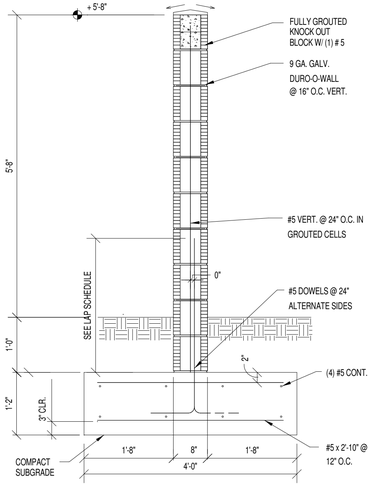
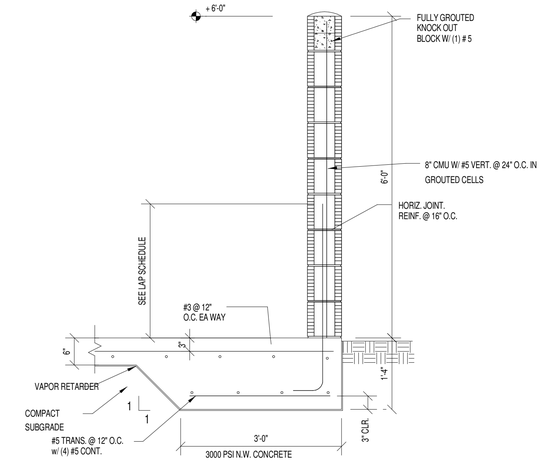


1 MONUMENT WALL

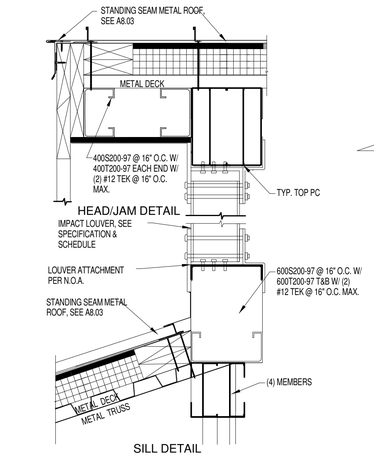
3/4" = 1'-0"

COORDINATE W/ ARCH. DWGS



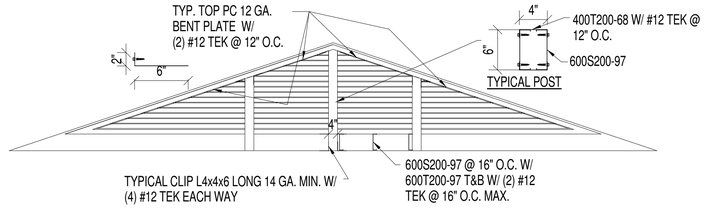
2 DUMPSTER WALL

3/4" = 1'-0"



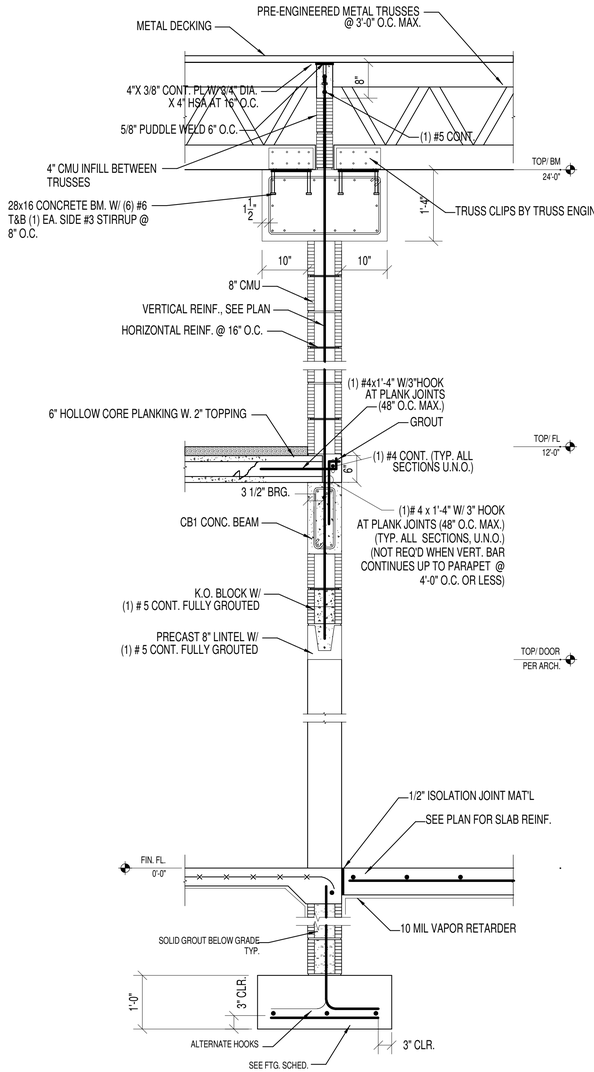
3 LOUVER DETAILS @ GABLE

1 1/2" = 1'-0"



4 GABLE END LOUVER ELEVATION

3/8" = 1'-0"

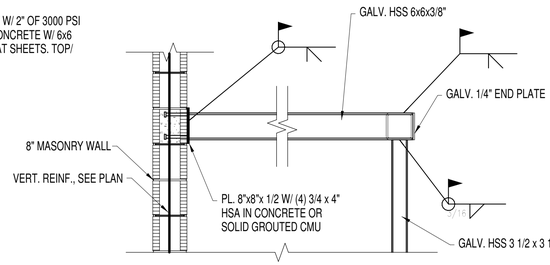


11 INTERIOR WALL SECTION

3/4" = 1'-0"

5 SECTION AT DECK

3/4" = 1'-0"

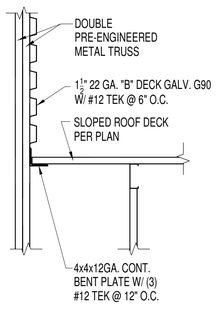


6 SECTION AT WALKWAY

3/4" = 1'-0"

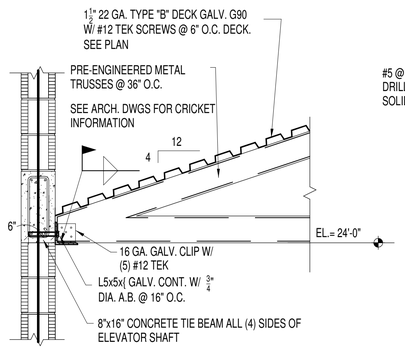
7 MASONRY OPENING ELEVATION

3/4" = 1'-0"



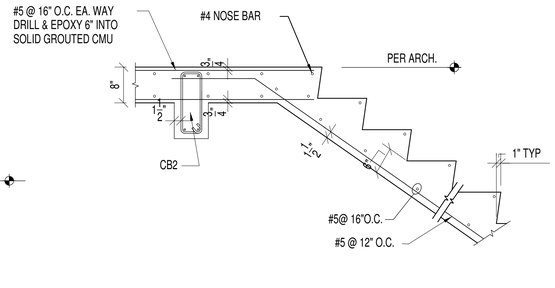
8 TRUSS SECTION

3/4" = 1'-0"



9 LOW TRUSS BEARING

3/4" = 1'-0"



10 STAIR SECTION

3/4" = 1'-0"

McCarthy and Associates, Inc.
CONSULTING ENGINEERS
www.mccarthyassoc.com
601 N. CONGRESS AVE. SUITE 106A
Delray Beach, FL. 33445
(561)265-6864
Florida Coa 4187
Robert J. Selinsky, P.E.
Florida P.E. 49952
McCarthy Project No. 11261

CURRIE
SOWARDS
AGUILA
ARCHITECTS

Architects, Planners
& Interior Designers

AA26001584
134 N.E. 1st Avenue
Delray Beach, Florida 33444
TEL: 561 276-4951
FAX: 561 243-8184
E-mail: Office@CSA-Architects.com

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DRC 11.29.2011
AAC 01.18.2012
AAC (RESUBMITTAL)..... 03.21.2012
P&Z 05.01.2012
BIDS.....08.06.2012
PERMIT.....08.06.2012
CONSTRUCTION .....

SEAL

PROJECT TITLE

CITY OF POMPANO BEACH FIRE STATION 103

3721 N.E. 12th AVE.
POMPANO BEACH, FL 33062

REVISIONS

THESE DRAWINGS ARE PREPARED PER ESTABLISHED INDUSTRY STANDARDS AND REPRESENT THE ARCHITECT AND ENGINEERS DESIGN CONCEPT. THEY ARE NOT INTENDED TO PROVIDE EVERY DETAIL OR CONDITION REQUIRED TO CONSTRUCT THE BUILDING. THE CONTRACTOR THROUGH SUBMITTALS AND OTHER COORDINATION EFFORTS IS FULLY RESPONSIBLE FOR PROVIDING A COMPLETE AND OPERATIONAL BUILDING WHETHER INDICATED ON THE PLANS OR NOT.

FILE NUMBER

DRAWING TITLE

STRUCTURAL DETAILS

08.06.2012 BID-PERMIT SET

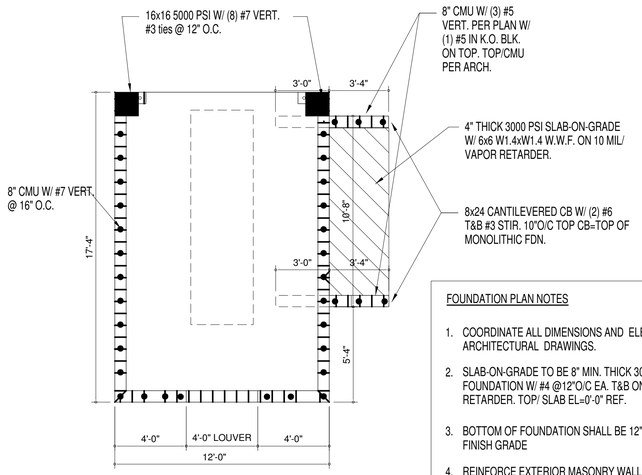
DATE 08.06.2012 | DRAWN BY NM

JOB NUMBER 110102

DRAWING NUMBER

S2.6

8/1/2012 1:54:22 PM

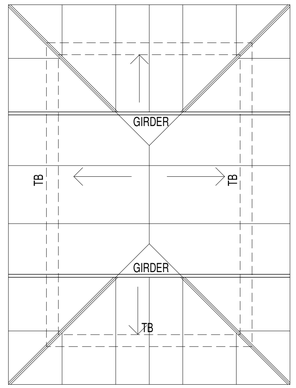


1 GENERATOR FLOOR PLAN

1/4" = 1'-0"

FOUNDATION PLAN NOTES

1. COORDINATE ALL DIMENSIONS AND ELEVATIONS WITH ARCHITECTURAL DRAWINGS.
2. SLAB-ON-GRADE TO BE 8" MIN. THICK 3000 PSI MONOLITHIC FOUNDATION W/ #4 @12"O/C EA. T&B ON 10 MIL VAPOR RETARDER. TOP/ SLAB EL=0'-0" REF.
3. BOTTOM OF FOUNDATION SHALL BE 12" MIN. BELOW FINISH GRADE
4. REINFORCE EXTERIOR MASONRY WALLS W/ #7 V. 16"O/C AND AT ALL CORNERS AND JAMBS U.NO. DOWEL OUT OF FDN. HOOK INTO TIE BM.

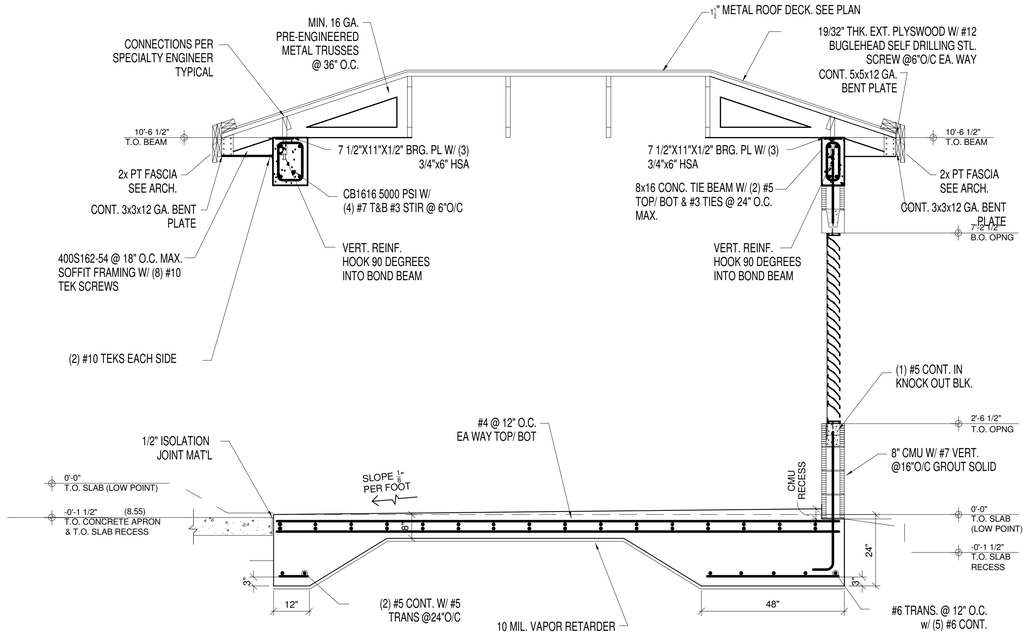


2 GENERATOR ROOF FRAMING PLAN

1/4" = 1'-0"

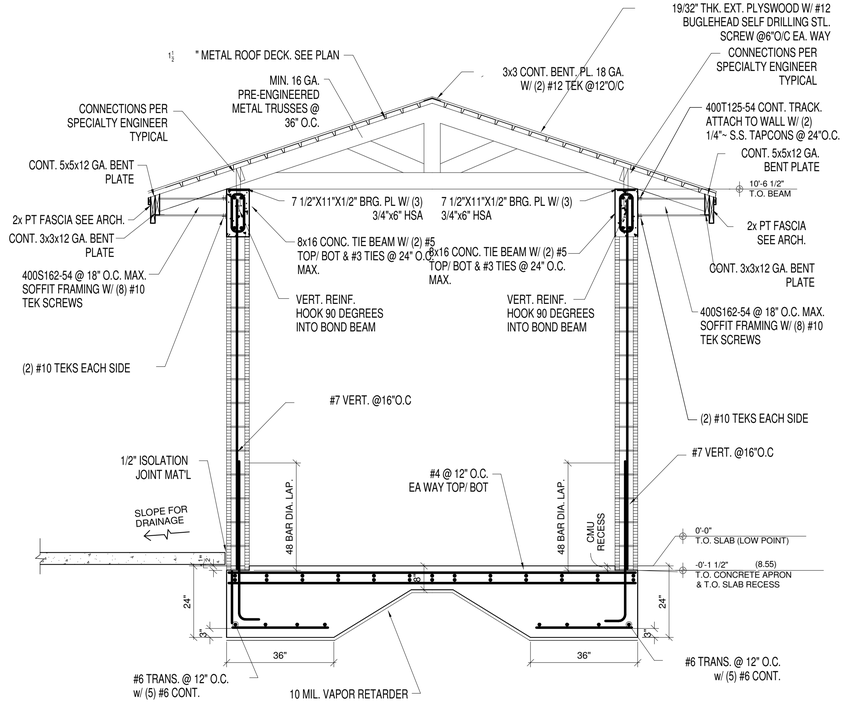
ROOF FRAMING PLAN NOTES

1. ROOF FRAMING TO BE PRE-ENGINEERED METAL TRUSSES EQUALLY SPACED BETWEEN SUPPORTS @ 36" O.C. U.N.O. PROVIDE BRIDGING PER MFR. TRUSS SPECIALTY ENGINEER.
2. METAL ROOF DECK TO BE 1 1/2" DP. (18) GA., GALVANIZED TYPE "B" INSTALLED (3) SPAN MIN. FASTEN DECK W/ 36/7 WELD PATTERN AT EACH SUPPORT AND (6) #10 TEK STITCH SCREWS SIDELAP FASTENERS FOR 36" WIDE DECKING.
3. ROOF SLOPE= 4/12 U.N.O. SEE ARCH. DWGS.
4. COORDINATE ALL DIMENSIONS/ELEVATIONS W/ ARCHITECT BEFORE STARTING CONSTRUCTION.
5. G.C./ ERECTOR IS RESPONSIBLE FOR ALL TEMPORARY BRACING.
6. REFER TO S2.0 FOR TYPICAL ROOF FRAMING DETAILS.
7. REFER TO SHEET S0.1, S0.2 & S0.3 FOR SPECIFICATIONS
8. TRUSS BEARING ELEVATION= (+) 10'-6-1/2"
9. GROSS UPLIFT FOR ROOFING MAT'LS =168 PSF
   NET UPLIFT FOR TRUSSES AND CONNECTIONS =156 PSF
10. TB= 8x16 4000 PSI W/ (2) #5 T&B #3 STIR. @24"O/C



3 GENERATOR LONGITUDINAL SECTION

1/2" = 1'-0"



4 GENERATOR CROSS SECTION

1/2" = 1'-0"

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GENERATOR FOUNDATION, ROOF PLAN & SECTIONS

08.06.2012 BID-PERMIT SET

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JOB NUMBER 110102

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